

DRAFT

Smoke Testing

1

Smoke Testing

Prepared for:

City of Bay City

SEW Consulting Engineers & Geologists, Inc.

275 Market Avenue
Coos Bay, OR 97420-2219
541/266-9890

November 2015
611013.151

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Reference: 611013.151

November 16, 2015

Mr. Brian Bettis
City of Bay City
PO Box 3309
Bay City, OR 97107

Subject: Smoke Testing Results, City of Bay City

Dear Brian:

SHN Consulting Engineers & Geologists, Inc. (SHN) performed smoke testing of the collection system for the City of Bay City (City) on August 24th-25th, 2015. Enclosed are the findings and recommendations, including two bound copies for your records and one reproducible copy for distribution to private property owners.

Please feel free to contact me at 541-266-9890 if you have any questions.

Sincerely,

SHN Consulting Engineers & Geologists, Inc.

A handwritten signature in black ink, appearing to read 'Steven K. Donovan', written over a light blue horizontal line.

Steven K. Donovan, PE
Principal Engineer

NJN:SKD:dkl

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Reference: 611013.151

Smoke Testing

Prepared for:

City of Bay City

PO Box 3309

Bay City, OR 97107

Prepared by:



Consulting Engineers & Geologists, Inc.

275 Market Avenue

Coos Bay, OR 97420-2228

541-266-9890

November 2015

QA/QC: SKD

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Acronyms and Abbreviations

City	City of Bay City
EPA	Environmental Protection Agency
SHN	SHN Consulting Engineers & Geologists, Inc.

1.0 Smoke Testing Procedure

Smoke testing is an Environmental Protection Agency (EPA) recommended procedure to identify flaws in a sewage system. To smoke test sewers, a motorized fan is placed over selected manholes forcing air through the pipes. A smoke bomb is placed on the suction side of the fan which allows the fan to inject smoke into the pipes. A properly functioning sewage system would dissipate the smoke out of the venting systems on the roofs of occupant's homes. Smoke that escapes in any other areas are general indications of a potential inflow source or as health hazards for occupants of homes without proper ventilation.

Smoke testing for the City of Bay City (City) was conducted on August 24th-25th of 2015. Conditions were moderate and sunny on both days. A total of 33 smoke bomb locations were tested for proper coverage of the entire town.

2.0 Smoke Testing Results

As shown on Figure 1, a total of 17 major problems and numerous minor problems were encountered during the test event. The test results located six major areas of inflow from structural damage in sewer pipe, manhole, or combined storm/wastewater cross connections. These areas of damage are highlighted with red arrows on the basin maps and should be the City's highest priority repair issues.

The second priority problem areas consist of broken laterals and roof drains connected to the sewer. These areas are highlighted in orange and could be significantly leading to inflow. There are 11 orange arrows located in three main areas. These must be fixed promptly as they can be large area drains and a major source of inflow during rainfall events. Because these problems reside on private property, the corrections may require the City to take enforcement action against the residents.

Finally, the yellow arrows mark uncapped cleanouts and venting issues that are not leading to a large portion of inflow but may be a health and safety concern for occupants. Improper venting can lead to sewer gasses within the house where occupants could be exposed.

Reports are attached in Appendix A for each of these areas describing the problem, where it is located, and a recommendation to fix the problem. A map of the North and South basins are also included in the report to show locations that may be experiencing high flows due to the inflow problems indicated.

2.1 List of No-Smoke Houses

These houses did not have smoke exiting their roofs meaning one of three possibilities:

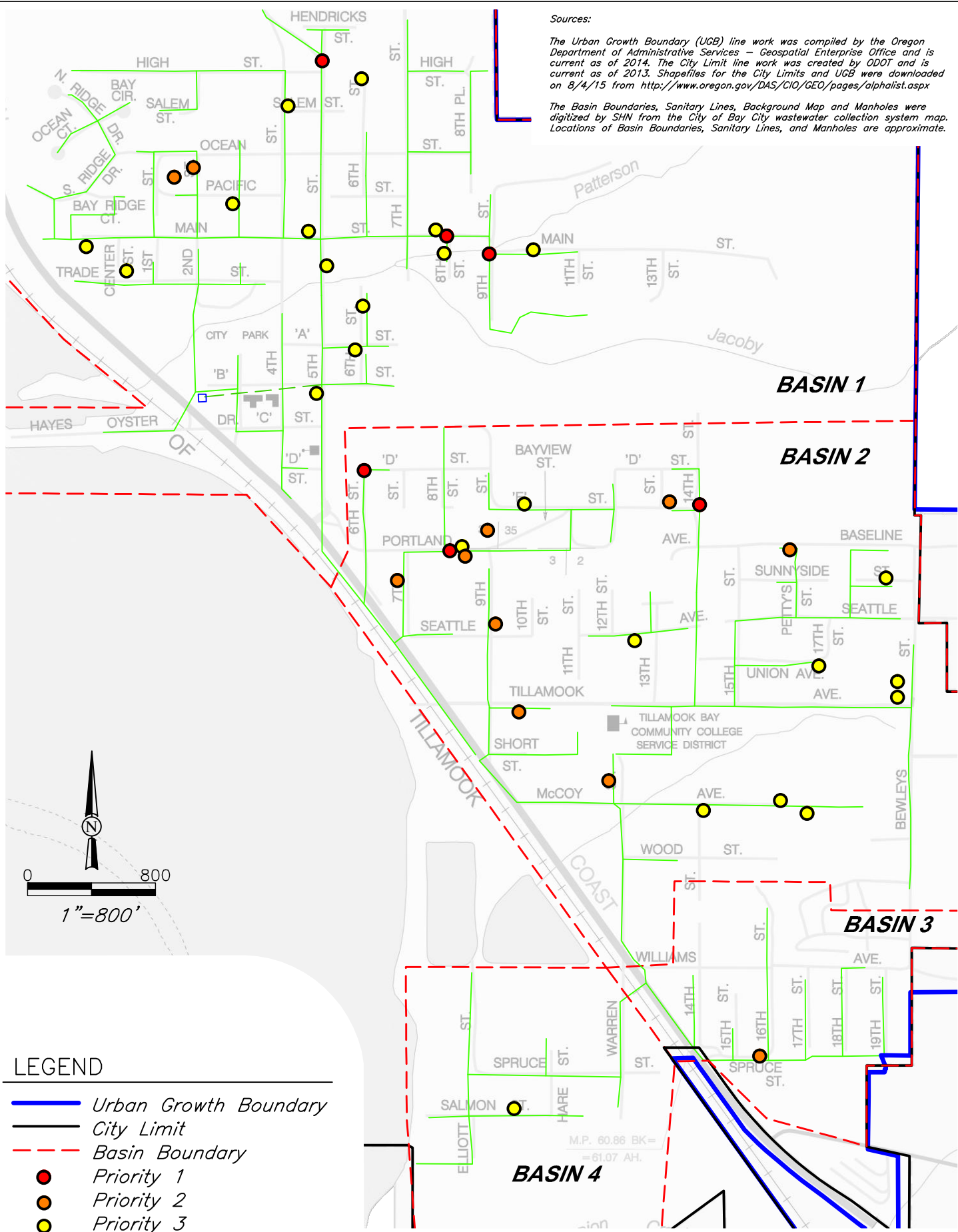
- They do not have their house vented or properly vented
- There is a sag in their lateral causing the smoke to be blocked.
- The sewage is being pumped uphill

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Sources:

The Urban Growth Boundary (UGB) line work was compiled by the Oregon Department of Administrative Services – Geospatial Enterprise Office and is current as of 2014. The City Limit line work was created by ODOT and is current as of 2013. Shapefiles for the City Limits and UGB were downloaded on 8/4/15 from <http://www.oregon.gov/DAS/CIO/GEO/pages/alpha1ist.aspx>

The Basin Boundaries, Sanitary Lines, Background Map and Manholes were digitized by SHN from the City of Bay City wastewater collection system map. Locations of Basin Boundaries, Sanitary Lines, and Manholes are approximate.



LEGEND

- Urban Growth Boundary
- City Limit
- - - Basin Boundary
- Priority 1
- Priority 2
- Priority 3

\\cooshay\svr1\Projects\2011\611013-Bay City EOR-Wastewater\151-WWFP\Figs , SA VED: 11/18/2015 3:46 PM NNISEN, PLOTTED: 11/19/2015 2:58 PM, NATHAN NISSEN



City of Bay City
 Wastewater Facilities Plan
 Bay City, Oregon

November 2015

Collection System Evaluation
 Smoke Testing Results
 SHN 611013

611013-SMOKE TEST RESULTS

Figure 1

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A fourth, less likely issue, could be that the lateral is blocked by roots or broken. If this were the case, the occupants would have a noticeably limited use of the plumbing. Caution must be taken when vactoring next to these properties due to the suction that the vactor creates forcing blowback into the plumbing of the house. This could cause sewage to splash back into the home and residents must be warned to close toilets until the vactor has passed.

Houses With No Sign of Smoke Exiting Roof Vents

- 9975 6th Ave.
- Duplex on 7th St.
- 9955 5th Ave.
- 10135 4th St.
- 10120 4th St.
- 5170 High St.
- 5035 S Ridge Dr.
- 9280 5th St.
- 9275 5th St.
- Pacific Oyster
- 12th St. Brown House
- 7990 18th St.
- 7965 19th St.
- 7850 19th St.
- 8595 Bewleys
- 8390 Bewleys
- 7860 Warren
- 4600 Salmon
- 4715 Salmon
- 5620 A St.
- 9670 Dewpoint
- 8970 15th St.
- 6880 Baseline
- 9435 5th St.
- 9075 12th
- 5195 Seattle
- 9930 3rd St.
- 7865 14th St.
- 7825 14th St.

In summary, there were 17 defects requiring correction to reduce extraneous flow. A document for each defect is attached to this report outlining where the defect is located and possible causes of the defect. A smoke testing result form is also attached that can be sent to property owners notifying them of the problem. Removal of inflow sources is the least expensive corrective measure to lower I/I in a sewer system. Not only is it economical, but removing these identified sources will have an exceptional impact on the reduction of I/I.

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A

Smoke Testing Results Forms

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SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 5970 Main St.
Owner: _____
Observer: Cody L.

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
In front of deck

Probable Cause
No cleanout cap

Recommendations
Replace broken cleanout cap.



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Photo No: 1

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: _____

Street Location: 8th Street Manhole
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

No smoke coming out of
manhole on 8th Street. Put
smoker on manhole at 8th
Street and Main.

Probable Cause

Dip or collapse in main

Recommendations

TV line to see what is
causing blockage



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Photo No: 2

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 9660 8th Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
No cap on cleanout

Probable Cause
Missing cleanout cap

Recommendations
Replace missing cap on cleanout.



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Photo No: 3

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 5680 Main Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Cleanout in demolished
house site.

Probable Cause
No cap on cleanout

Recommendations
Repair broken cleanout



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Photo No: 4

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: Cleanout at end of 6th St.
Owner: _____
Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Cap is open on cleanout

Probable Cause
Open Cap

Recommendations
Close cap on cleanout.



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Photo No: 5

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: _____
Time: _____

Street Location: West side of 5th
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Leak north of Ocean Street
before Hendricks Street

Probable Cause
Unknown

Recommendations
Check area for cleanout and
TV pipe.



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Photo No: 6

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 2nd Street east of 5365
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke coming out of
ground next to power pole
guy wire.

Probable Cause
Possible roots from stump.

Recommendations
Inspect pipe



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Photo No: 7

Project No. 611013
Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: Empty lot on 1st Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
To the east side of 1st Street
At 1st and Pacific.

Probable Cause
Too brushy to determine
Location of smoke source.
Possible cleanout

Recommendations
Inspect area for cleanout



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Photo No: 8

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 9590 6th Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Black pipe on side of house

Probable Cause
Vent comes out on the side of the house.

Recommendations
Not a problem



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Photo No: 9

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 9360 5th Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Smoke through bathroom
sink.

Probable Cause

No trap in sink

Recommendations

Install trap



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Photo No: 10

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: E St. between 9th & 11th
Owner: _____
Observer: Cody L.

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Hole in ground.

Probable Cause
Cleanout with no lid.

Recommendations
Replace lid on cleanout.



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Photo No: 11

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: Between 8th & 9th
Owner: _____
Observer: Cody L.

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Manhole with AC patch.
Trench patch has smoke
Leaking out of cracks. Also,
2" PVC pipe with smoke
between houses 6050 8th &
6075 8th.

Probable Cause
Cracks in manhole
Casing.

Recommendations
Repair/replace manhole and
or sealant



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Photo No: 12

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: **Bay City**

Street Location: Empty lot next to 6075
8th Street

Date: 8-25-2015

Owner: _____

Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Hole with smoke coming out
of ground

Probable Cause

Broken lateral

Recommendations

Repair lateral



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Photo No: 13

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: Manhole at 7th & D St
Owner: _____
Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke leaking out of
Manhole rim on the side.

Probable Cause
Sealant is missing

Recommendations
Seal manhole



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Photo No: 14

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: _____
Time: _____

Street Location: 8855 9th Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Coming up in yard. All of piping
up to cleanout is smoking heavily
(bricks, concrete, planter box,
and retaining wall)

Probable Cause

Lateral is broken in multiple
places

Recommendations

Replace lateral



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Photo No: 15

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6425 Seattle Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Probable Cause
Broken cleanout cap.

Recommendations
Replace cleanout cap.



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Photo No: 16

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 8870 Bewley Street
Owner: _____
Observer: Cody L.

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
1/2 way down Sunny Side
10' off of the road. Leak
could be cleanout

Probable Cause
Broken cleanout cap.

Recommendations
Replace cleanout cap.



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Photo No: 17

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6795 McCoy
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Probable Cause
No cleanout lid.

Recommendations
Cap cleanout.



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Phone: 541-266-9890

Photo No: 18

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6780 McCoy
Owner: _____
Observer: Cody L.

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Probable Cause
No cleanout lid.

Recommendations
Replace cleanout lid.



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275 Market Avenue
Coos Bay, OR 97420
Phone: 541-266-9890

Photo No: 19

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: _____
Time: _____

Street Location: 6190 Main Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Square roof vent on northwest
Side leaking smoke.

Probable Cause
Not properly vented

Recommendations
Vent through roof



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Phone: 541-266-9890

Photo No: 27

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 9th and Main Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Water coming into main
from stream culvert that runs
under 9th

Probable Cause
Crack in main

Recommendations
Repair main



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Phone: 541-266-9890

Photo No: 28

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6085 Portland Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke coming out of gutter.

Probable Cause
Gutter hooked up to sewer

Recommendations
Separate gutter and sewer



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Photo No: 35

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 8945 7th Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke out of lawn on south
side of house.

Probable Cause
Broken lateral

Recommendations
Replace lateral



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Phone: 541-266-9890

Photo No: 36

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6525 E. Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Ditch is leaking in front of
house across from manhole.

Probable Cause
Broken lateral

Recommendations
Repair lateral



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Phone: 541-266-9890

Photo No: 37

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6765 Baseline
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke exiting out of water
meter box.

Probable Cause
Lateral leaking by water box

Recommendations
Inspect water box and lateral
repair if necessary



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Photo No: 38

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 8680 Bewley
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

Probable Cause

Cleanout not capped.

Recommendations

Replace cap on cleanout.



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Phone: 541-266-9890

Photo No: 39

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6625 McCoy
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke coming out of shop
roofline.

Probable Cause
Vent exiting roof or not
properly vented

Recommendations
Vent through roof



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Photo No: 40

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6395 Short Street
Owner: _____
Observer: Nate Nissen

LOCATION OF RETURN (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke exiting vent on south-
east side of house. Also
exiting gutter line on the
south side of the house

Probable Cause
Gutters draining into
cleanout

Recommendations
Separate gutter and cleanout



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Coos Bay, OR 97420
Phone: 541-266-9890

Photo No: 41

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 9635 5th Street
Owner: _____
Observer: Walter White

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Vent pipe does not go above
eave.

Probable Cause
Vent pipe does not go above
eave.

Recommendations
Vent around eave.



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Photo No: 52

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 10035 4th Street
Owner: _____
Observer: Walter White

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke coming out of
crawlspace under house

Probable Cause
Not properly vented/
Possible broken lateral

Recommendations
Inspect lateral and vent



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Photo No: 53

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-24-2015

Street Location: 5415-5515 Pacific
Owner: _____
Observer: Walter White

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke from 4" PVC
in empty lot

Probable Cause

Recommendations
Cap cleanout



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Photo No: 56

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City

Street Location: 9635 Trade

Date: 8-24-2015

Owner: _____

Observer: Walter White

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

No cleanout cover south
side of house.

Probable Cause

Recommendations

Cap cleanout



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Photo No: 57

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
 Date: 8-25-2015

Street Location: 6195 Tillamook
 Owner: _____
 Observer: Walter White

LOCATION OF RETURN
 (location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
 Broken cleanout cover on the
 West side. Smoke from under
 House and cracks in sidewalk
 NE corner of house.

Probable Cause
 Broken lateral

Recommendations
 Repair lateral



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Photo No: 61

Project No. 611013

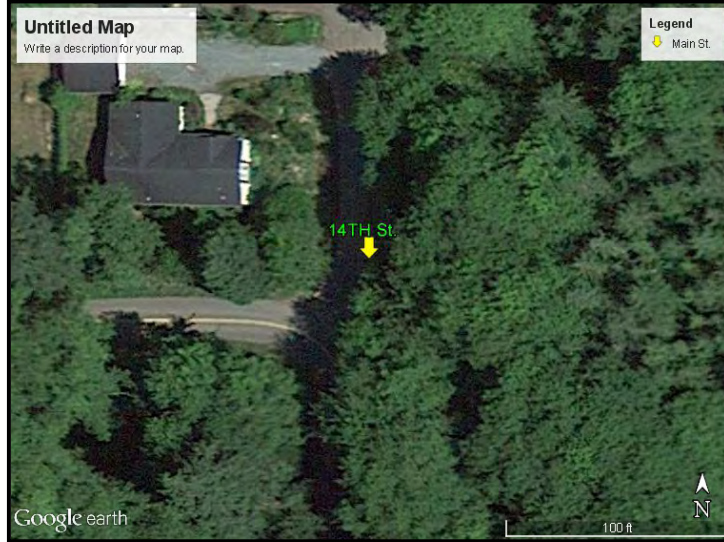
Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: SE Corner E & 14th St.
Owner: _____
Observer: Walter White

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Manhole leaking out of side
ring.

Probable Cause
No sealant around ring.

Recommendations
Manhole needs to be sealed.



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Phone: 541-266-9890

Photo No: 64

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6825 Union
Owner: _____
Observer: Walter White

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
No cleanout cover on the
North side - old home site
East of existing house.

Probable Cause

Recommendations
Cap cleanout



Consulting Engineers & Geologists, Inc.
275 Market Avenue
Coos Bay, OR 97420
Phone: 541-266-9890

Photo No: 65

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City

Street Location: 8630 Bewely

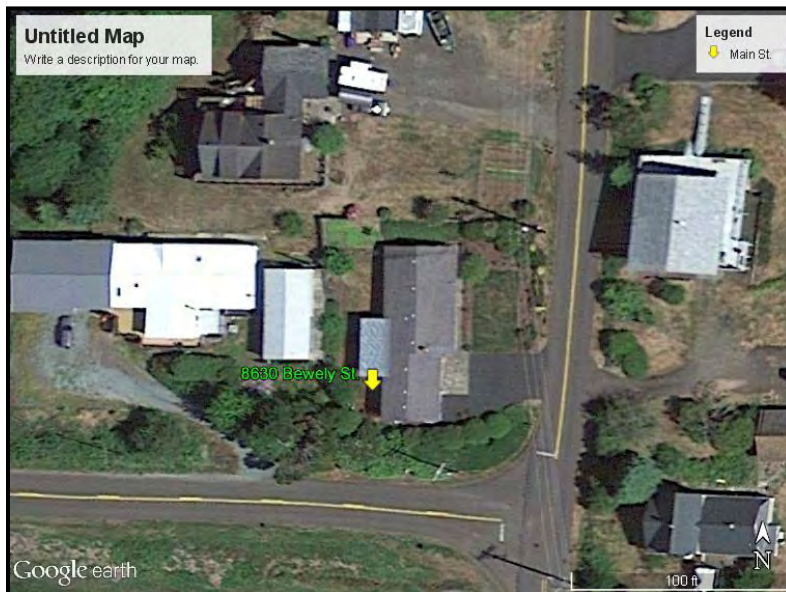
Date: 8-25-2015

Owner: _____

Observer: Walter White

LOCATION OF RETURN

(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return

No cleanout cover on the
west side of house.

Probable Cause

Recommendations

Cap cleanout



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275 Market Avenue

Coos Bay, OR 97420

Phone: 541-266-9890

Photo No: 66

Project No. 611013

Task: 151

SMOKE TESTING RESULTS

Client: Bay City
Date: 8-25-2015

Street Location: 6760 Spruce St.
Owner: _____
Observer: Brian Bettis

LOCATION OF RETURN
(location of MH, street, house no., areas of smoke escape, photo, etc.)



Observed Smoke Return
Smoke exiting gutter

Probable Cause
Gutter hooked to cleanout

Recommendations
Separate gutter from
cleanout



Consulting Engineers & Geologists, Inc.

275 Market Avenue

Coos Bay, OR 97420

Phone: 541-266-9890

Photo No: 67

Project No. 611013

Task: 151

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Sample Notification Form Letter

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City of Bay City
P.O. Box 3309
Bay City, OR 97107
(503)-377-2179

Date_____

Owner_____

Address_____

City, State_____

Subject Property_____

Dear Property Owner:

The Wastewater Treatment Plant experiences extremely high flows during the winter months. This can, in large part, be attributed to “holes” in the sewage collection and piping system including laterals to properties served by the system. In an effort to locate these holes and reduce the high seasonal inflows, the City of Bay City recently undertook a smoke testing project in your neighborhood. The project included pumping smoke into manholes and observing where the smoke escapes from the system. If smoke is observed leaving the sewer system through a “hole,” surface and/or groundwater is capable of entering the system through the same “hole.” The potential for one of these infiltration “holes” was discovered on your property and requires your attention to correct the problem.

Some of the problems discovered are directly related to the infiltration waters that overload the sewer system during the winter months. Other problems are related to plumbing deficiencies outside the home, which should be corrected.

A side benefit of the smoke testing project was that, in some cases, smoke was observed not venting from homes properly. While this could be a result of a sag or unused element in the household plumbing, it could also represent a potential health risk. If a household sewer system is not functioning properly, harmful sewer gases may find their way into the house. This type of plumbing deficiency should be corrected immediately.

The following sheet includes a checklist of potential problems discovered during the smoke testing project. If a problem is marked with an X, it requires the action described immediately after the marked description.

If, for some reason, you are unable to correct the problem in the time suggested, please contact the City. We are interested in correcting these problems and will help in any way we can to do that.

1. ____ MAY NOT HAVE A PERMITTED SEWER CONNECTION ON RECORD.
Please contact the City to discuss this matter.
2. ____ RVS OR ADDITIONAL HOOK-UP INTO SEWER SYSTEM.
Notification is hereby given to remove.
3. ____ PIPING OR LATERAL PIPE PROBLEMS ON SITE.
Have plumbing inspection by qualified person. Report result to City within four (4) weeks of this notice.
4. ____ RAIN GUTTERS CONNECTED TO SEWER SYSTEM.
Immediate removal of roof drains from sewer system required. City personnel will be on site within two (2) weeks of the date of this notice to inspect the outfall of the roof drain system to confirm disconnection.
5. ____ AREA DRAIN OR OTHER SURFACE DRAINAGE SYSTEM TIED INTO SEWER SYSTEM.
Immediate removal of area drains from sewer system required. City personnel will be on site within two (2) weeks of the date of this notice to inspect the area drain to confirm disconnection.
6. ____ UNCAPPED OR OPEN SEWER LATERAL CLEANOUT.
Immediate cap of lateral cleanout required with watertight cap. City personnel will be on site within four (4) weeks of the date of this notice to inspect the cleanout to confirm capping.
7. ____ SMOKE INSIDE HOUSE OR BUILDING.
Have inspection and repairs performed by qualified person. Sewer gas passing into the home can pose a serious health risk.
8. ____ OTHER PROBLEM.

Please note that any of these problems are of a serious nature. Any items marked with an X require your immediate attention and cooperation. Please call the City at 503-377-2179 if you have any questions. By reducing these high seasonal inflows to the sewer system, we can help reduce unnecessary sewer treatment costs, maintain the highest levels of sewer system performance, and keep our sewer rates as low as possible.

Thank you for your help in this matter.

Sincerely,

City of Bay City

DRAFT

Cleaning – TV Results 2

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City of Bay City
TV/Cleaning Results
November 2015

Manhole #	Feet	Pipe Size	Description	Comments	Total feet Televised
02-01		15"	Reinforced Concrete Pipe	Elliot Street	
@	0.0		Access Point - Manhole	Upstream MH 02	0.0
@	362.5		Access Point - Manhole	Downstream MH 01	362.5
03-02		15"	Reinforced Concrete Pipe	Elliot Street	
@	0.0		Access Point - Manhole	Upstream MH 03	0.0
@	37.4	4"	Tap-Factory Made	Service Left @ 9:00	37.4
@	293.7		Access Point - Manhole	Downstream MH 02	293.7
04-03		15"	Reinforced Concrete Pipe	Spruce Street	
@	0.0		Access Point - Manhole	Upstream MH	0.0
@	54.1	4"	Tap-Factory Made	Service Right @ 2:00	54.1
@	58.1	4"	Tap-Factory Made	Service Left @ 9:00	58.1
@	115.1	4"	Tap-Break-in/Hammer	Service Left @ 10:00	115.5
@	162.8	4"	Tap-Factory Made	Service Left @ 9:00	162.8
@	186.7	4"	Tap-Factory Made	Service Right @ 3:00	186.7
@	238.2	4"	Tap-Factory Made	Service Left @ 9:00	238.2
@	317.8	4"	Tap-Factory Made	Service Left @ 9:00	317.8
@	320.0	4"	Tap-Factory Made	Service Left @ 9:00	320.0
@	464.6		Access Point - Manhole	Downstream MH 03	464.6
05-04		15"	Reinforced Concrete Pipe	Spruce Street	
@	0.0		Access Point - Manhole	Upstream MH	0.0
@	85.5	4"	Tap-Factory Made	Service Right @ 3:00	85.5
@	173.7	4"	Tap-Factory Made	Service Left @ 9:00	173.7
@	192.1	4"	Tap-Factory Made	Service Right @ 3:00	192.1
@	245.9	4"	Tap-Factory Made	Service Right @ 3:00	245.9
@	253.8	4"	Tap-Factory Made	Service Left @ 9:00	253.8
@	291.8	4"	Tap-Factory Made	Service Right @ 3:00	291.8
@	316.1	4"	Tap-Factory Made	Service Right @ 3:00	316.1
@	490.8		Access Point - Manhole	Downstream MH 04	490.8
06-05		15"	Reinforced Concrete Pipe	Warren Street	
@	0.0		Access Point - Manhole	Upstream MH 06	0.0
@	28.3	4"	Tap-Factory Made	Service Right @ 2:00	28.3
@	158.3	4"	Tap-Break-in/Hammer	Service Right @ 1:00	158.3
@	168.4	4"	Tap-Factory Made	Service Left @ 10:00	168.4
@	189.2	4"	Tap-Factory Made	Service Left @ 9:00	189.2
@	211.3	4"	Tap-Factory Made	Service Left @ 9:00	211.3
@	283.8	4"	Tap-Factory Made	Service Right @ 3:00	283.8
@	338.8	4"	Tap-Factory Made	Service Left @ 9:00	338.8
@	503.0		Access Point - Manhole	Downstream MH 05	503.0
07-06		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 07	0.0
@	170.3		Access Point - Manhole	Downstream MH 06	170.3

City of Bay City
TV/Cleaning Results
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Manhole #	Feet	Pipe Size	Description	Comments	Total feet Televised
07A-07		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 07A	0.0
@	72.3		Access Point - Manhole	Downstream MH 07	72.3
08-07		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 08	0.0
@	31.6		Access Point - Manhole	Downstream MH 07	31.6
08A-08		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 08A	0.0
@	34.9		Access Point - Manhole	Downstream MH 08	34.9
09-08A		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 09	0.0
@	4.5		Access Point - Manhole	Downstream MH 08A	4.5
10-09		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 10	0.0
@	253.2	4"	Tap-Factory Made	Service Left @ 9:00	253.2
@	415.5	4"	Tap-Factory Made	Service Left @ 10:00	415.5
	510.8		Access Point - Manhole	Downstream MH 09	510.8
11-10		15"	Reinforced Concrete Pipe	Warren Street	
@	0.0		Access Point - Manhole	Upstream MH 11	0.0
@	76.1	4"	Tap-Factory Made	Service Right @ 3:00	76.1
@	130.5	4"	Tap-Factory Made	Service Left @ 9:00	130.5
@	331.2		Access Point - Manhole	Downstream MH 10	331.2
12-11		15"	Reinforced Concrete Pipe	Warren Street	
@	46.5		Access Point - Manhole	Downstream MH 12	46.5
@	0.0		Access Point - Manhole	Upstream MH 11	0.0
12A-12		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 12A	0.0
@	45.6	4"	Tap-Factory Made	Service Left @ 9:00	45.6
@	160.3	4"	Tap-Factory Made	Service Left @ 9:00	160.3
@	286.1		Access Point - Manhole	Downstream MH 12	286.1
13A-12A		15"	Reinforced Concrete Pipe	McCoy Ave	
@	0.0		Access Point - Manhole	Upstream MH 13A	0.0
@	41.8	4"	Tap-Factory Made	Service Left @ 9:00	41.8
@	102.5	4"	Tap-Factory Made	Service Left @ 9:00	102.5
@	168.6	4"	Tap-Factory Made	Service Right @ 3:00	168.6
@	216.9	4"	Tap-Factory Made	Service Left @ 9:00	216.9
@	223.0	4"	Tap-Factory Made	Service Right @ 3:00	223.0
@	305.4		Access Point - Manhole	Downstream MH 12A	305.4
13A-13		15"	Pre-stressed Concrete Cylinder Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 13A	0.0
@	217.8	4"	Tap-Factory Made	Service Left @ 10:00	217.8
@	289.7		Access Point - Manhole	Downstream MH 13	289.7

City of Bay City
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Manhole #	Feet	Pipe Size	Description	Comments	Total feet Televised
14B-13		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 14B	0.0
@	345.8		Access Point - Manhole	Downstream MH 13	345.8
14C-13		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 14C	0.0
@	66.1		Access Point - Manhole	Downstream MH 13	66.1
14-14B		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 14	0.0
@	95.6		Access Point - Manhole	Downstream MH 14B	95.6
15A-15		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 15A	0.0
@	438.0	4"	Tap-Factory Made	Service Left @ 10:00	438.0
@	447.6		Access Point - Manhole	Downstream MH 15	447.6
16-15		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 16	0.0
@	168.3		Infiltration-Runner	at 6:00	168.3
@	231.1	4"	Tap-Factory Made	Left @ 10:00	231.1
@	395.4		Access Point - Manhole	Downstream MH 15	395.4
17-16		15"	Reinforced Concrete Pipe	US Highway 101	
@	0.0		Access Point - Manhole	Upstream MH 17	0.0
@	435.2		Access Point - Manhole	Downstream MH 16	435.2
18-17		15"	Reinforced Concrete Pipe	5th Street	
@	0.0		Access Point - Manhole	Upstream MH 18	0.0
@	72.6		Infiltration-Runner	at 10:00	72.6
@	94.3		Access Point - Manhole	Downstream MH 17	94.3
21-20		15"	Reinforced Concrete Pipe	5th Street	
@	0.0		Access Point - Manhole	Upstream MH 21	0.0
@	99.8		Tap-Factory Made	Service Left @ 9:00	99.8
@	104.2		Tap-Factory Made	Service Right @ 3:00	104.2
@	162.0		Tap-Factory Made	Service Right @ 3:00	162.0
@	191.0		Tap-Factory Made	Service Left @ 9:00	191.0
@	326.7		Tap-Factory Made	Service Right @ 3:00	326.7
@	347.0		Tap-Factory Made	Service Right @ 3:00	347.0
@	360.5		Tap-Factory Made	Service Left @ 9:00	360.5
@	389.0		Tap-Factory Made	Service Left @ 3:00	389.0
@	447.1		Tap-Factory Made	Service Left @ 9:00	447.1
@	488.0		Tap-Factory Made	Downstream MH 20	488.0
20-19		12"	Reinforced Concrete Pipe	5th Street	
@	0.0		Access Point - Manhole	Upstream MH 20*	0.0
@	300.0		Access Point - Manhole	Downstream MH 19*	300.0

* Upstream MH 20 = in front of post office. Downstream HM 19 = bottom of 5th Street

City of Bay City
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Manhole #	Feet	Pipe Size	Description	Comments	Total feet Televised
33-32A		10"	Reinforced Concrete Pipe	14th Street	
	0.0		Access Point-Manhole	Upstream MH 33	0.0
@	32.2	4"	Tap-Factory Made-Capped	Service Right @ 2:00	32.2
@	79.5	4"	Tap-Factory Made-Capped	Service Right @2:00	79.5
@	106.7		Access Point-Manhole	Downstream MH 32A	106.7
40-40A		8"	Reinforced Concrete Pipe	Spruce Street	
	0.0		Access Point-Manhole	Upstream MH 40	0.0
@	74.1	6"	Tap-Factory Made	Service Left @ 9:00	74.1
@	109.3	6"	Tap-Factory Made	Service Right @ 3:00	109.3
@	175.3		Infiltration-Stain	From 10:00 to 7:00	175.3
@	204.7		Access Point-Manhole	Downstream MH 40A	204.7
40A-33		8"	Reinforced Concrete Pipe	Spruce Street	
	0.0		Access Point-Manhole	Upstream MH 40A	0.0
@	52.1		Infiltration - Dripper	at 3:00	52.1
@	92.3	4"	Tap-Factory Made	Service Left @ 9:00	92.3
@	151.9	6"	Tap-Factory Made	Service Right @ 3:00	151.9
@	156.6	6"	Tap, Break-in/Hammer	Service Right @ 3:00 Plugged	156.6
@	199.0		Infiltration - Dripper	at 2:00 Deposits Attached:	199.0
@	220.5		Infiltration - Dripper	at 3:00 Encrustation 5% @2:00	220.5
@	220.6		Access Point-Manhole	Downstream MH 33 outside Drop	220.6
41-40		8"	Reinforced Concrete Pipe	Spruce Street	
	0.0		Access Point-Manhole	Upstream MH 41	0.0
@	31.8	6"	Tap, Break-in/Hammer	Service Right @ 3:00	31.8
@	34.4	4"	Tap, Break-in/Hammer	Service Left @ 9:00	34.4
@	66.9	6"	Tap, Factory Made	Service Right @ 2:00	66.9
@	95.4	6"	Tap, Break-in/Hammer	Service Right @ 9:00	95.4
@	98.4	6"	Tap-Factory Made	Service Left @ 9:00	98.4
@	119.3	6"	Tap-Factory Made	Service Right @ 3:00	119.3
@	123.9	6"	Tap, Break-in/Hammer	Service Left @ 9:00	123.9
@	151.3	6"	Tap, Break-in/Hammer	Service Left @ 9:00	151.3
@	220.1	6"	Tap-Factory Made	Service Left @ 9:00	220.1
@	229.6		Access Point-Manhole	Downstream MH 40	229.6
41A-41		8"	Reinforced Concrete Pipe	17th Street	
	45.1		Access Point-Manhole	Upstream MH 41A	45.1
@	0.0		Access Point-Manhole	Downstream MH 41	0.0
42-41		8"	Reinforced Concrete Pipe	18th Street	
	0.0		Access Point-Manhole	Upstream MH 42	0.0
@	41.7	4"	Tap, Break-in/Hammer	at 10:00	41.7
@	43.0	4"	Tap-Factory Made	Service Left @ 10:00	43.0
@	73.0	6"	Tap, Break-in/Hammer	Service Right @ 9:00	73.0
@	86.1	4"	Tap-Factory Made	Service Right @ 3:00	86.1
@	158.6	6"	Tap-Factory Made		
@	158.6		Infiltration- Gusher	Service Right @ 2:00 Infiltration in the Lateral @6:00 Gusher	158.6
@	185.0		Access Point-Manhole	Downstream MH 41	185.0

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43-42		8"	Reinforced Concrete Pipe	19th Street	
@	261.6		Access Point-Manhole	Upstream MH 43	261.2
@	182.2	4"	Tap, Break-in/Hammer	Service Right @ 2:00	182.2
@	158.0	6"	Tap-Factory Made	Service Left @ 10:00	158.0
@	124.3	4"	Tap, Break-in/Hammer	Service Right @ 2:00 Pipe Broken @ 4:00	124.3
@	99.2	6"	Tap-Factory Made	at 10:00	99.2
@	44.0	4"	Tap, Break-in/Hammer	at 2:00	44.0
@	36.0		Infiltration - Stain	at 2:00	36.0
@	20.9		Infiltration - Stain	at 3:00	20.9
@	19.4		Infiltration - Stain	at 2:00	19.4
@	0.4	2"	Tap, Break-in/Hammer	Service Right @ 2:00 pipe Intruding 2"	0.4
@	0.0		Access Point-Manhole	Downstream MH 42	0.0
52-79		8"	Reinforced Concrete Pipe	Tillamook Ave	
@	0.0		Access Point-Manhole	Upstream MH 52	0.0
@	92.7	6"	Tap-Factory Made	Service Left @ 9:00	92.7
@	225.1		Access Point-Manhole	Downstream MH 79	225.1
53-52		8"	Reinforced Concrete Pipe	15th Street	
	0.0		Access Point-Manhole	Upstream MH 53	0.0
@	82.4	8"	Tap-Factory Made	at 9:00	82.4
@	166.1	8"	Tap-Factory Made	Service Right @ 3:00	166.1
@	284.9		Access Point-Manhole	Downstream MH 52	284.9
54-53		8"	Reinforced Concrete Pipe	15th Street	
	0.0		Access Point-Manhole	Upstream MH54	0.0
@	8.9	6"	Tap-Factory Made	Service Right @ 3:00	8.9
@	38.7	4"	Tap, Break-in/Hammer	Service Right @ 2:00 Capped	38.7
@	134.4	6"	Tap-Factory Made	Service Right @ 3:00 Capped	138.4
@	184.3		Access Point-Manhole	Downstream MH 53	184.3
55A-54		8"	Reinforced Concrete Pipe	15th Street	
	0.0		Access Point-Manhole	Upstream MH 55A	0.0
@	84.7		Access Point-Manhole	Downstream MH 54	84.7
55-54		8"	Reinforced Concrete Pipe	Seattle Ave	
	0.0		Access Point-Manhole	Upstream MH 55	0.0
@	6.1	6"	Tap-Factory Made	Service Left @ 9:00	6.1
@	16.4	6"	Tap-Factory Made	Service Right @ 3:00	16.4
@	58.4	6"	Tap-Factory Made	Service Left @ 9:00	58.4
@	166.5	4"	Tap, Break-in/Hammer	Service Right @ 1:00	166.5
@	290.5		Access Point-Manhole	Downstream MH 54	290.5
56-55		8"	Reinforced Concrete Pipe	Seattle Ave	
	0.0		Access Point-Manhole	Upstream MH 56	0.0
@	3.2		Water Mark 40% (diameter)		3.2
@	15.4		Infiltration - Stain	at 10:00	15.4
@	100.5		Access Point-Manhole	Downstream MH 55	100.5
57-56		8"	Reinforced Concrete Pipe	Seattle Ave	
	0.0		Access Point-Manhole	Upstream MH 57	0.0
@	72.7		Access Point-Manhole	Downstream MH 56	72.7

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58-57		8"	Reinforced Concrete Pipe	Seattle Ave	
	0.0		Access Point-Manhole	Upstream MH 58	0.0
@	44.7		Infiltration - Stain	at 2:00	44.7
@	113.5	6"	Tap, Break-in/Hammer	Service Right @ 3:00	113.5
@	236.8	6"	Tap, Break-in/Hammer	Service Right @ 3:00	236.8
@	270.6		Access Point-Manhole	Downstream MH 57	270.6
59-58		8"	Reinforced Concrete Pipe	Seattle Ave	
	0.0		Access Point-Manhole	Upstream MH 59	0.0
@	92.3	6"	Tap-Factory Made	Service Right @ 3:00	92.3
			Infiltration Stain in Lateral on		
			right side		
@	96.7		Infiltration - Stain	from 6:00 to 11:00	96.7
@	130.8	6"	Tap-Factory Made	Service Left @ 9:00	130.8
@	156.8		Access Point-Manhole	Downstream MH 58	156.8
64-56		8"	Reinforced Concrete Pipe	Sunnyside Street	
	0.0		Access Point-Manhole	Upstream MH 64	0.0
@	67.0	4"	Tap-Factory Made	Service Left @ 10:00	67.0
@	105.4	6"	Tap-Factory Made	Service Right @ 2:00	105.4
@	163.5		Infiltration - Stain	at 9:00	163.5
@	168.1	4"	Tap-Factory Made	Service Right @ 3:00	168.1
@	218.4		Infiltration - Runner	at 10:00	218.4
@	220.1		Access Point-Manhole	Downstream MH 56	220.1
65-64		8"	Reinforced Concrete Pipe	Sunnyside Street	
	127.2		Access Point-Manhole	Upstream MH 65	127.2
@	108.7	4"	Tap-Factory Made	Service Left @ 9:00	108.7
@	0.0		Access Point-Manhole	Downstream MH 64	0.0
66-65		8"	Reinforced Concrete Pipe	Portland Ave	
	0.0		Access Point-Manhole	Upstream MH 66	0.0
@	74.6	6"	Tap, Break-in/Hammer	Service Right @ 2:00	74.6
@	86.0		Access Point-Manhole	Downstream MH 65	86.0
153-145		8"	Reinforced Concrete Pipe	7th Street	
	0.0		Access Point-Manhole	Upstream MH 153	0.0
@	6.2		Deposits Settled: Compacted	from 5:00 to 7:00	6.2
@	45.5	6"	Tap-Factory Made	Service Right @ 3:00	45.5
@	49.0	6"	Tap-Factory Made	Service Left @ 9:00	49.0
@	115.5	6"	Tap-Factory Made	Service Left @ 9:00	115.5
@	213.2	6"	Tap-Factory Made	Service Right @ 3:00	213.2
@	221.1		Water Level: Sag	10% (diameter)	221.1
@	312.4		Surface: Aggregate Projecting	from 7:00 to 4:00	312.4
@	328.1	4"	Tap, Break-in/Hammer	Service Left @ 10:00	328.1
			Pipe Intruding 2"		
@	405.4	4"	Tap-Factory Made	Service Left @ 9:00	405.4
@	412.3	6"	Tap-Factory Made	Service Right @ 3:00	412.3
@	478.9		Point Repair - Pipe Replaced	Pipe Replaced	478.9
@	508.7		Infiltration - Runner	at 10:00	508.7
@	510.6		Access Point-Manhole	Downstream MH 145 Outside	510.6
			drop		

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154-153	8"	Reinforced Concrete Pipe	7th Street	
	0.0	Access Point-Manhole	Upstream MH 154	0.0
@	76.6	6" Tap-Factory Made	Service Right @ 3:00	76.6
@	85.1	Point Repair - Pipe Replaced	Pipe Replaced Settled to cause offset joint	85.1
@	87.1	4" Tap-Factory Made	Service Right @ 9:00	87.1
@	118.7	6" Tap-Factory Made	Service Left @ 9:00-Capped	118.7
@	160.7	8" Tap-Factory Made	Service Right @ 3:00	160.7
@	185.3	6" Tap-Factory Made	Service Left @ 9:00-Capped	185.3
@	188.3	Surface: Missing Aggregate at 6:00		188.3
@	201.1	Point Repair - Pipe Replaced	Pipe Replaced Settled to cause offset joint	201.1
@	202.7	4" Tap-Factory Made	at 9:00	202.7
@	204.5	Infiltration - Dripper	at 2:00	204.5
@	208.8	Hole in pipe: soil visible	at 12:00	208.8
@	220.1	6" Tap-Factory Made	Service Left @ 9:00-Capped	220.1
@	325.1	6" Tap-Factory Made	at 9:00	325.1
@	328.5	4" Tap-Factory Made	at 3:00	328.5
@	330.3	Point Repair - Pipe Replaced	Pipe repair settled	330.3
@	332.0	4" Tap-Factory Made	Service Left @ 9:00. Heavy infiltration in the Lateral	332.0
@	405.6	6" Tap-Factory Made	Service Left @ 9:00-Capped	405.6
@	417.8	Point Repair - Pipe Replaced	Pipe Replaced Settled to cause offset joint	417.8
@	419.9	4" Tap-Factory Made	Service Left @ 10:00	419.9
@	423.4	6" Tap, Break-in/Hammer	Service Left @ 9:00	423.4
@	447.2	4" Tap-Factory Made	Service Right @ 3:00	447.2
@	501.4	Point Repair - Localized Lining	Local Lining	501.4
@	505.1	Access Point-Manhole	Downstream MH 153	505.1
155-154	8"	Reinforced Concrete Pipe	7th Street	
	0.0	Access Point-Manhole	Upstream MH 155	0.0
@	80.3	6" Tap, Break-in/Hammer	Service Right @ 3:00	80.3
@	119.0	Roots, Tap: Barrel Crack Circumferential	at 8:00 from 7-4	119.0
@	119.7	6" Tap, Break-in/Hammer	Service Right @ 3:00	119.7
@	120.6	Roots, Fine: Barrel	From 8:00-4:00	120.6
@	140.1	4" Tap, Break-in/Hammer	Service Right @ 10:00	140.1
@	216.7	8" Tap-Factory Made	Service Right @ 3:00	216.7
@	266.8	6" Tap, Break-in/Hammer	Service Right @ 10:00	266.8
@	313.3	Crack Circumferential Infiltration - Stain	From 7:00-11:00 From 7:00-11:00	313.3
@	315.1	Access Point-Manhole	Downstream MH 154	315.1

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**Mixing Zone Report -
Draft**

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Mixing Zone Report

City of Bay City
PO Box 3309
Bay City, OR 97107



Prepared for:

City of Bay City



September 2018

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Reference: 611013.151

Mixing Zone Report

Prepared for:
City of Bay City
PO Box 3309
Bay City, OR 97107

Prepared by:



Engineers & Geologists
275 Mark Avenue
Coos Bay, OR 97420
541-266-9890

August 2018

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Abbreviations and Acronyms

m/s	
mg/L	
pH	
ppt	parts per thousand
ADWF	Average Dry Weather Flow
AOU	Apparent Oxygen Utilization
AWWF	Average Wet Weather Flow
BOD	Biological Oxygen Demand
CCC	Criteria Chronic Concentration
CMC	Criteria Maximum Concentration
DEQ	Oregon Department of Environmental Quality
DO	Dissolved Oxygen
EPA	Environmental Protection Agency
FONSI	Finding of No Significant Impact
NPDES	National Pollutant Discharge Elimination System
SBR	Sequential Batch Reactor
SHN	SHN Engineers & Geologists
TDZ	Toxic Dilution Zone
US EPA	United States Environmental Protection Agency
ZID	Zone of Immediate Dilution

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1.0 Introduction

1.1 Purpose

The City of Bay City is required by NPDES permit 101025 to conduct a periodic review of its outfall and conduct a mixing zone analysis based on current information. During the course of this work, it was determined that the City's existing outfall was failing and becoming inundated by bay sediments. A new outfall and mixing zone was determined necessary. The study was modified to evaluate new outfall locations and to evaluate and optimize an outfall design configuration for the City of Bay City's Wastewater Treatment Plant effluent disposal system. Based on the study results, an outfall design and mixing zone located lower in the estuary system are recommended to provide the City with a long-term effluent disposal system that is both environmentally and financially acceptable. The proposed project site, shown in Figure 1 is located the North Coast Basin of Western Oregon west of the City of Bay City in Tillamook Bay.

1.2 Objective

This study applied research on ambient conditions and a computer model to simulate discharge scenarios and predict how much dilution will occur in an area defined as the "mixing zone". The results of the study are intended to demonstrate that, outside of the mixing zone, the recommended outfall design will meet all regulatory criteria under all foreseeable ambient conditions and foreseeable future uses.

Predictions of dilution consider two critical regulatory areas within the mixing zone:

The zone of initial dilution where acute criteria (Criteria Maximum Concentration) must be achieved and

The fringe of the mixing zone where all water quality criteria including chronic criteria (Criteria Continuous Concentration) must be achieved.

Criteria, which define adequate mixing in these two locations and an analysis of the dilution of the parameters of concern for each criterion, are considered in this study.

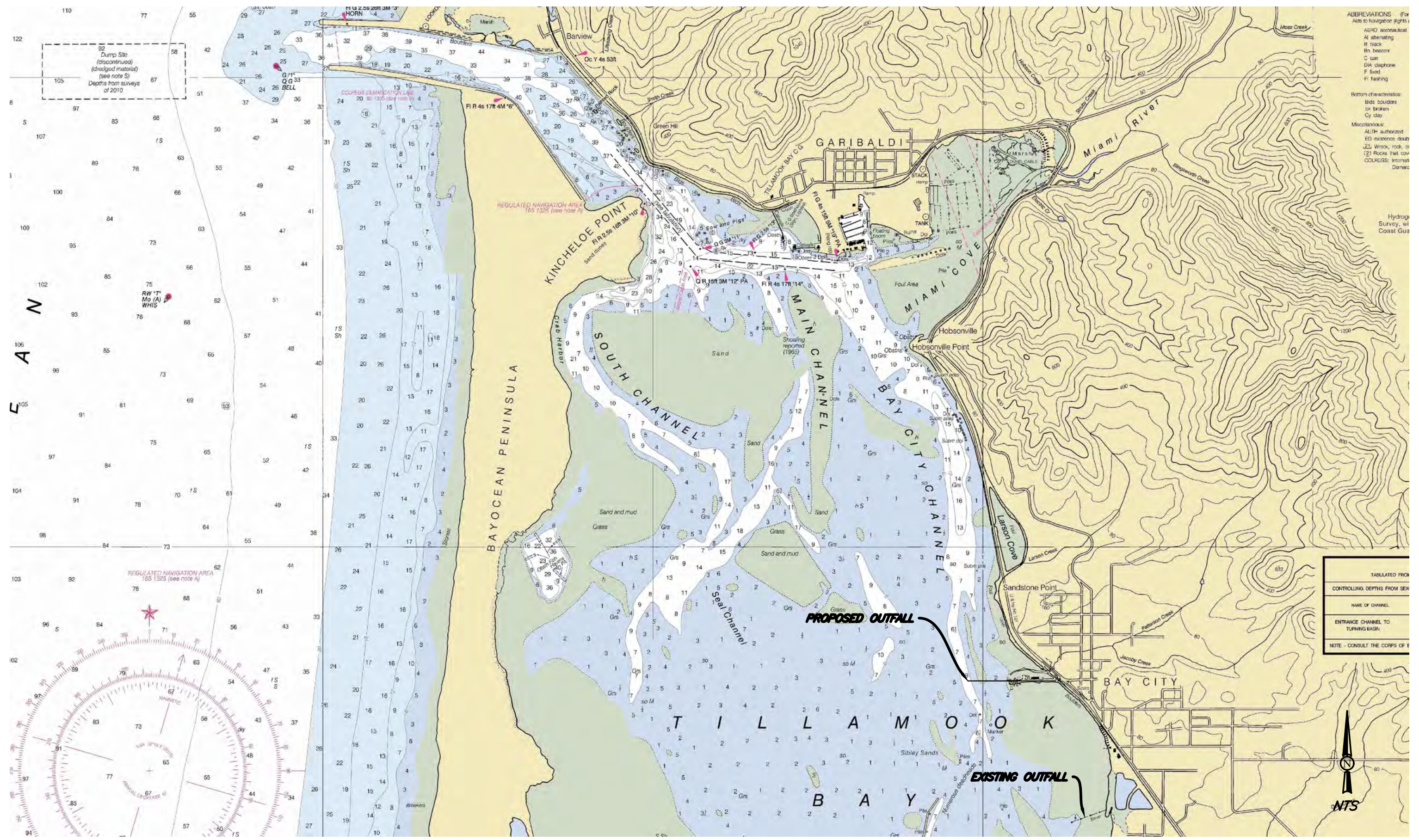
1.3 Scope of Work

This report includes the following items:

- Field reconnaissance and an evaluation of the existing outfall.
- A summary of past studies and data collected to characterize ambient conditions that affect the discharge and effluent disposal system design.
- Conducting an analysis of outfall designs using the Environmental Protection Agency's (EPA) CORMIX, version GI mixing zone model.
- Modeling different outfall diffuser configurations to optimize the mixing achieved from the disposal system.

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ABBREVIATIONS (For Aids to Navigation lights)


AERO aeronautical
 A alternating
 B buoy
 Bn beacon
 C can
 DIA diaphane
 F fixed
 F flashing

Bottom characteristics:
 Bds boulders
 bk broken
 Cy clay

Miscellaneous:
 AUTH authorized
 ED existence doubtful
 JZJ Whisk, rock, or
 (2) Rocks (not cov)
 COLREGS International
 Damaged

Hydrog Survey, w/ Coast Gu

TABULATED FROM
CONTROLLING DEPTHS FROM SEA
NAME OF CHANNEL
ENTRANCE CHANNEL TO TURNING BASIN
NOTE - CONSULT THE CORPS OF E

 Consulting Engineers & Geologists, Inc.	CITY OF BAY CITY WASTEWATER FACILITIES PLAN Bay City, Oregon	BAY CITY PROPOSED WW OUTFALL SITE PLAN SHN 010000
	January 2010	611013-OFALL

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- Preparing a mixing zone study report that presents the background data, mixing zone modeling results, and recommendations for the design of a diffuser structure.]

The City’s mixing zone study should identify an optimum diffuser design, demonstrate that dilution effect in the ambient water mitigate effluent pollutants, and support a finding of no significant impact (FONSI), concerning continued disposal of the City of Bay City’s wastewater effluent to the Tillamook estuary.

2.0 Principal Rules and Regulations

Water Quality Standards for the North Coast basin are set forth in OAR 340-041-0230 and are enforced by the Oregon Department of Environmental Quality (DEQ). Specific water quality standards not to be exceeded for wastewater discharge are determined for marine waters. Primary regulations of concern are provided below:

OAR [340-041-0230](#)

Basin-Specific Criteria (North Coast): Beneficial Uses to Be Protected in the North Coast Basin

- (1) Water quality in the North Coast Basin must be managed to protect the designated beneficial uses shown in Table 230A (November 2003). (see Appendix 1)
- (2) Designated fish uses to be protected in the North Coast Basin are shown in Figures 230A and 230B (November 2003). (see Appendix 1)
- (3) Coastal water contact recreation use is to be protected in all North Coast Basin marine waters and in coastal waters designated in Figures 230C through 230H (August 2016). (see Appendix 1)
- (4) Shellfish harvesting use is to be protected in all North Coast Basin marine waters and in coastal waters as designated in Figures 230C through 230H (August 2016). (see Appendix 1)

[ED. NOTE: To view tables referenced in rule text, [click here to view rule.](#)]

Statutory/Other Authority: ORS 468.020, 468B.030, 468B.035 & 468B.048

Statutes/Other Implemented: ORS 468B.030, 468B.035 & 468B.048

History:

DEQ 9-2016, f. & cert. ef. 8-18-16

DEQ 17-2003, f. & cert. ef. 12-9-03

OAR [340-041-0234](#)

Basin-Specific Criteria (North Coast): Approved TMDLs in the Basin:

The following TMDLs have been approved by EPA, and appear on the Department’s web site:

Nestucca Bay Drainage — Temperature, Bacteria and Sediment — May 13, 2002

Tillamook Bay Drainage — Temperature and Bacteria — July 31, 2001

North Coast — Temperature and Bacteria — August 20, 2003

Statutory/Other Authority: ORS 468.020, 468B.030, 468B.035 & 468B.048

Statutes/Other Implemented: ORS 468B.030, 468B.035 & 468B.048

History:

DEQ 17-2003, f. & cert. ef. 12-9-03

OAR [340-041-0235](#)

Basin-Specific Criteria (North Coast): Water Quality Standards and Policies for this Basin

- (1) pH (hydrogen ion concentration). pH values may not fall outside the following ranges:

- (a) Marine waters: 7.0–8.5;
 - (b) Estuarine and fresh waters: 6.5–8.5.
 - (2) Total Dissolved Solids. Guide concentrations may not be exceeded unless otherwise specifically authorized by DEQ upon such conditions as it may deem necessary to carry out the general intent of this plan and to protect the beneficial uses set forth in OAR 340-04I-0230: All Fresh Water Streams and Tributaries (other than the main stem Columbia River) — 100.0 mg/l.
 - (3) Minimum Design Criteria for Treatment and control of Sewage Wastes in this Basin:
 - (a) During periods of low stream flows (approximately April 1 to October 31): Treatment resulting in monthly average effluent concentrations not to exceed 20 mg/l of BOD and 20 mg/l of SS or equivalent control;
 - (b) During the period of high stream flows (approximately November 1 to April 30): A minimum of secondary treatment or equivalent control and unless otherwise specifically authorized by the Department, operation of all waste treatment and control facilities at maximum practicable efficiency and effectiveness so as to minimize waste discharges to public waters.
- Statutory/Other Authority:** ORS 468.020, 468B.030, 468B.035 & 468B.048
Statutes/Other Implemented: ORS 468B.030, 468B.035 & 468B.048
History:
 DEQ 2-2007, f. & cert. ef. 3-15-07
 DEQ 17-2003, f. & cert. ef. 12-9-03

2.1 Application of Regulatory Criteria

North coast basin rules pertaining to the City of Bay City discharge are within the design capabilities of the wastewater treatment plant. Effluent quality requirements can be met provided the City achieves adequate secondary treatment at the facility. Some allowances for toxic pollutants may be necessary to account for a toxic dilution zone for ammonia. Chlorine discharge will not be considered since the City will be utilizes a UV disinfection system.

2.2 Pollutants of Concern

The significant pollutants of concern, reasons for concern, and probable concentration limits are summarized in the Table 1 below. Ammonia represents the toxic substance that must be addressed in a toxic dilution zone.

Table 1 Summary of Significant Pollutants of Concern

Pollutant of Concern	Reason for Concern	Water Quality Criteria
Ammonia at end of toxic dilution zone	Acute Toxicity	Temp & pH dependent
Ammonia at Fringe of Mixing Zone	Chronic Toxicity	Temp & pH dependent
Chlorine at end of toxic dilution zone	Acute Toxicity	0.013 mg/L (not) ¹
Chlorine at Fringe of Mixing Zone	Chronic Toxicity	0.0075 mg/L (not) ¹
Temperature at Fringe of Mixing Zone	Biological Life	+/- 0.2°C
Turbidity at Fringe of Mixing Zone	Aesthetics	< 10% Increase
DO at Fringe of Mixing Zone	Biological Life	+/- 0.1 mg/L
Geometric Mean of Bacteria at End of Pipe	Human exposure	126 E. coli org./100 ml
Geometric Mean of Bacteria at Fringe of Mixing Zone	Human exposure	14 E. coli org./100 ml

1. Not present in effluent discharge.

Ammonia concentrations for chronic and acute toxicity limits at the anticipated range of temperatures and pH for a salinity of 30 kg/L as defined by EPA's Ambient Water Quality Criteria for Ammonia (Saltwater) - 1989 are summarized in Table 2 below.

Table 2 Water Quality Criteria for Saltwater Aquatic Life Based on Total Ammonia Concentration as N, (mg/L)

Temperature	11.8°C		14.4°C		17.0°C	
	Acute	Chronic	Acute	Chronic	Acute	Chronic
pH 7.6	25.38	3.81	21.31	3.20	17.65	2.65
pH 8.2	6.52	0.98	5.50	0.83	4.58	0.69
pH 8.9	1.45	0.22	1.25	0.19	1.04	0.16

Data developed from DEQ Ammonia Calculator Based on EPA 440/5-88-004 April 1989

2.3 Mixing Zones

A mixing zone is an established area where water quality standards may be exceeded as long as acute toxic conditions are prevented and the states designated beneficial uses are protected. Generally, regulatory criteria apply at the fringe of the mixing zone.

2.3.1 Regulatory Mixing Zone

The RMZ at the Bay City outfall must be redefined. Currently the outfall provides no mixing during events.

2.3.2 Zone of Immediate Dilution (ZID)

In accordance with OAR 340-041-0325.4.b, allowances for exceeding acute criteria may be necessary in a zone of immediate dilution (ZID) in order to allow dilution of toxic constituents to below acute criteria (in other words, the CMC). The EPA in Technical Support Document for Water quality-Based Toxics Control provides guidance for setting stringent criteria that can be used to limit the ZID (referenced by EPA as a toxic dilution zone or TDZ) based on probable exposure with no impact from short-term contact with toxic constituents. Three criteria are provided, the more stringent of which should govern the limit of the ZID.

1. The CMC should be met at a distance of 10 percent of the distance from the edge of the outfall structure to the edge of the regulatory mixing zone.
2. The CMC should be met within a distance of 50 times the discharge length scale in any special direction. (The discharge length scale is calculated as the square root of the discharge port area).
3. The CMC should be met within a distance of five times the local water depth in any horizontal direction from any discharge outlet.

Each of these criteria should be considered in defining the mixing zone and ZID for the Bay City outfall.

3.0 Ambient Conditions of Tillamook Bay

Tillamook Bay is a small, shallow estuary about 60 miles west of Portland on the Oregon Coast. Tillamook Bay, along with other Oregon estuaries, is a truly a unique environments. This environment is influenced by numerous and complex estuarine factors that affect the mixing process. Approximately 6.2 miles long and 2.1 miles wide, the Bay averages only 6.6 feet depth. At low tide, about 50% of the bottom is exposed as intertidal mud flats.

In addition to ambient forces, the design of the outfall diffuser will have an initial influence on the transport and spreading of effluent. Design criteria are discussed later in this report and are based on an understanding of how ambient conditions, primary currents, and turbulence control the transport and mixing phenomenon farther from the discharge. This section describes the research, analytical tools, and background data gathered to describe ambient conditions.

3.1 Physical Setting

The proposed outfall site, shown in Figure 1, is to be located approximately 4,500 feet northwest of the existing outfall, in the upper reach of the Bay City channel, on the eastern side of mid bay, between Sandstone point and Goose Point. This area of the bay was selected as a probable outfall location due to the proximity of a main channel (that holds water during low tide), proximity to the WWTP, and the location's distance from environmental receptors.

3.2 Environmental Mapping

“Environmental mapping” has been prepared to identify the areas in and around the proposed outfall area that may be sensitive to the impact of the discharge on beneficial uses within the bay. Habitats, critical resources areas, and other beneficial uses are mapped within the segment of the water body receiving the discharge.

Established water quality standards require management of water quality to protect beneficial uses, which fall into the following categories:

- Designated fish uses to be protected in the Bay
- Shellfish harvesting
- Coastal water contact recreation

3.2.1 Fish Use

The bay provides habitat for numerous fish, shellfish, crabs, birds, seals and sea grasses. Multiple species of fish have been identified in the bay at various times of the year. Five species of anadromous salmon use the bay at some point in their life cycle. The Tillamook Watershed is home to Summer and Winter Steelhead, Coho, Chum, Spring and Fall Chinook and sea-run Cutthroat Trout. The following fish species resident in the bay are federally listed as “Threatened” under the Endangered Species Act:

- Coho salmon
- Green sturgeon

- Eulachon (commonly called smelt, candlefish, or hooligan)

None of these species spawn in the bay, but use the bay for rearing and migration. Water quality is to be managed in order to accommodate salmon and trout rearing and migration within the waters of the Bay. In addition to threatened species, Oregon also lists the Pacific lamprey as a State Species of Concern and Steelhead are listed as a federal Species of Concern. Figure 2 depicts fish use areas in the Bay (<http://www.oregonfishinginfo.com/Tillamook%20Bay.html>).

3.2.3 Shellfish Harvesting Use

Clam digging and crabbing are important for the economy and lifestyle within the Tillamook watershed. Oysters have been grown commercially in Tillamook Bay since the 1930's. Tillamook Bay has been one of the leading oyster producing bays in Oregon, with an average annual production of about 21,200 shucked gallons during the 1970s and 1980s. Beginning in 1990, the level of production dropped off sharply and has remained low due to reduced production by several Oyster Companies. Figure 3 depicts the oyster growing lease areas in the Bay. 2016 shellfish plat production was 5,926.69 gallons of shucked oysters in Tillamook Bay, (Source: Oregon Department of Agriculture, Natural Resources Program). Figure 4 depicts clamming and crabbing use areas in the Bay.

3.2.2 Recreational Use

Water contact recreational use of the estuary is typically limited to activities associated with sport fishing and shellfish harvesting. Figure 4 generally depicts the location of public boating access points and recreational shellfish harvesting areas within the bay.

3.3 Existing Outfall Site

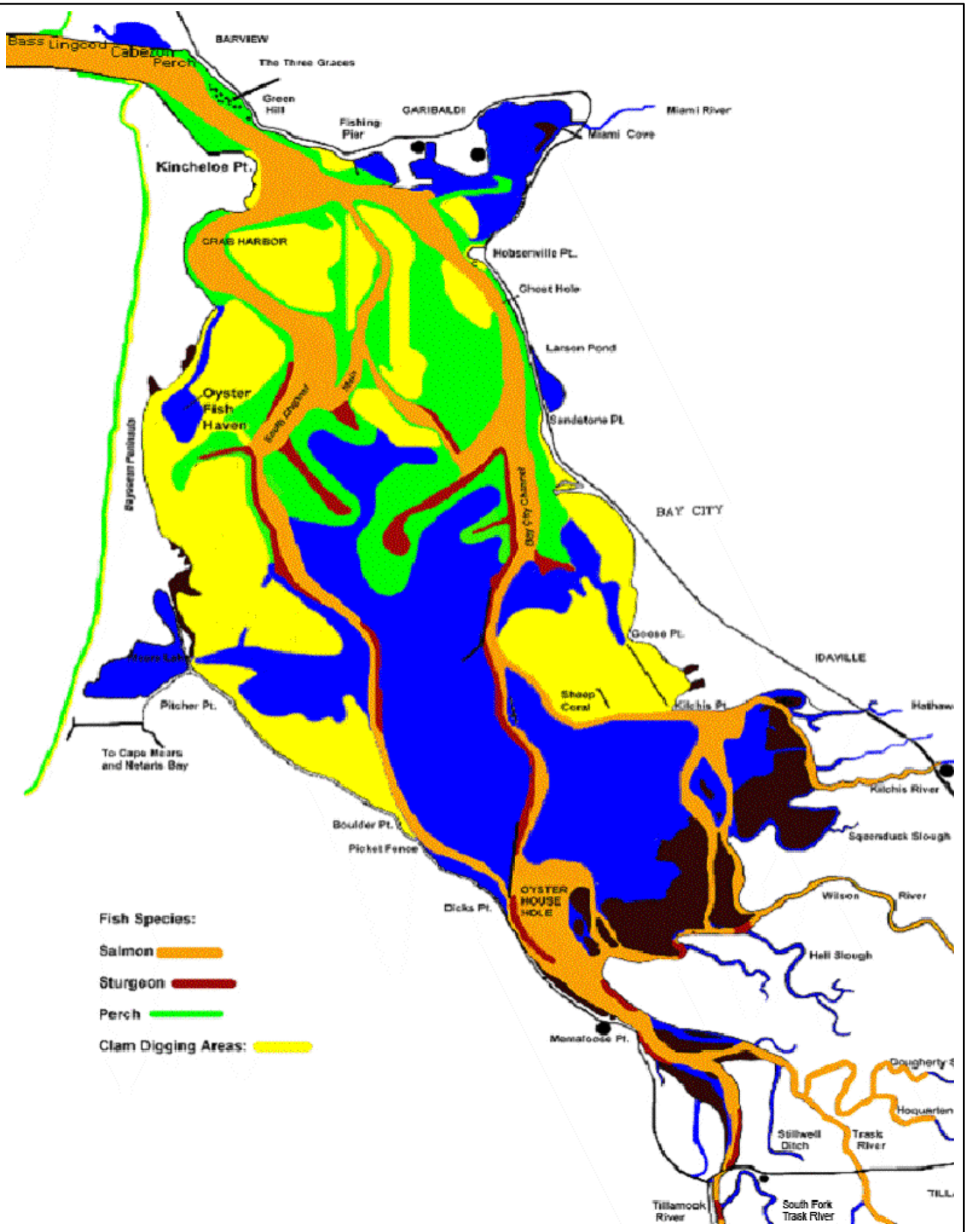
The existing outfall is located approximately 2,000 feet north of Goose Point on the east side of the Bay. The outfall pipe extends approximately 1,250 feet from the eastern shoreline into the Bay, situated in what was once a shallow channel, serving Doty Creek. The Doty Creek channel, when the outfall was planned and installed, was approximately 2-3 deep at Mean Low Water. Storm events within the area have relocated that channel closer to the shore line and the outfall diffuser is currently inundated with sediment and discharges in a "bubble-up" fashion into adjacent mud-flats. When exposed at lower tides, effluent flows across the mud flats as it makes its way back to the channel.

3.4 Proposed Outfall Site

Due to the location of the existing outfall site being in the mud flats and observed channel migration, a new outfall will need to be located in the Tillamook estuary. The proposed outfall site, (Figure 1), is to be located approximately 4,500 feet northwest of the existing outfall, in the upper reach of the Bay City channel, on the eastern side of mid bay, between Sandstone point and Goose Point. This location is intended to situate the

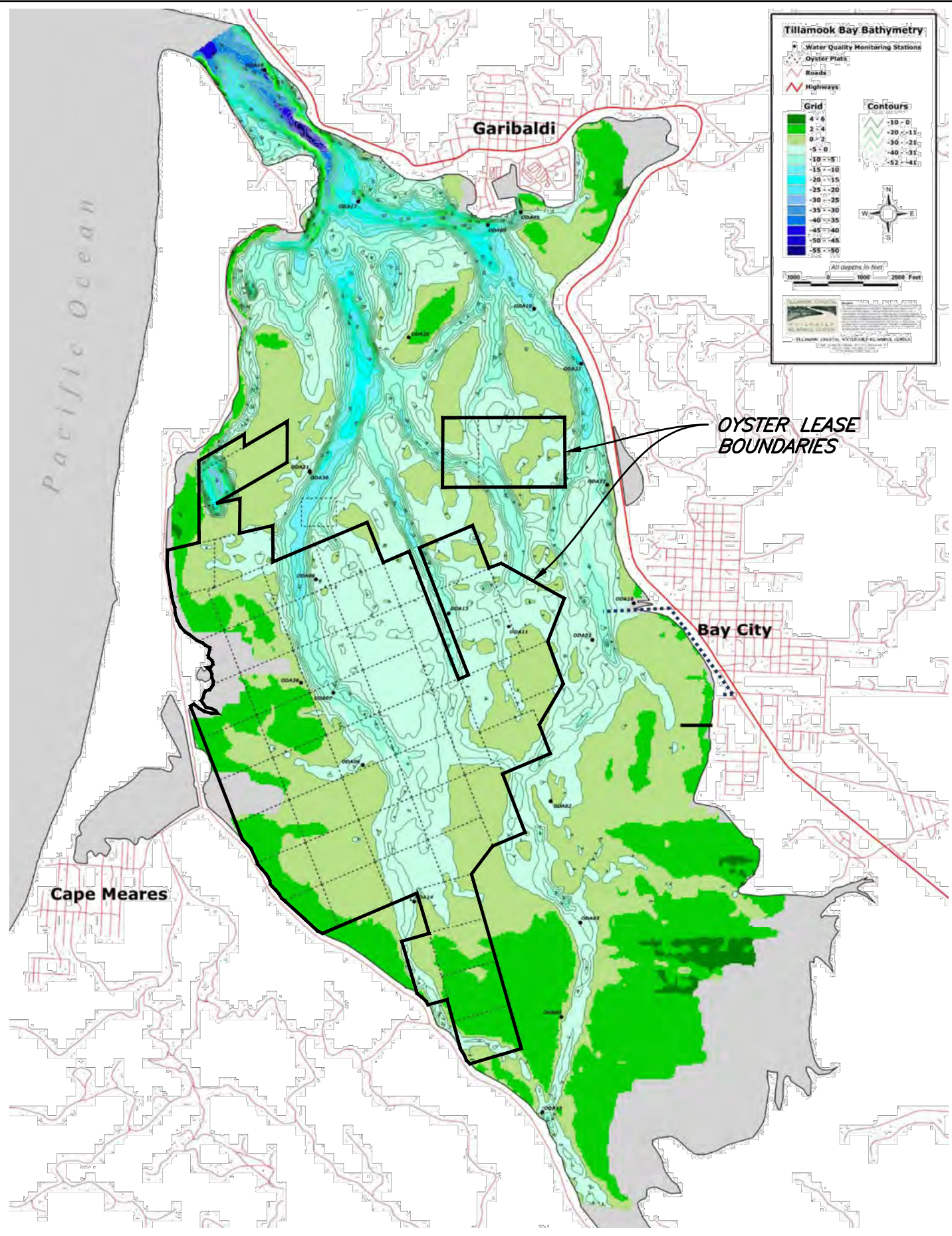
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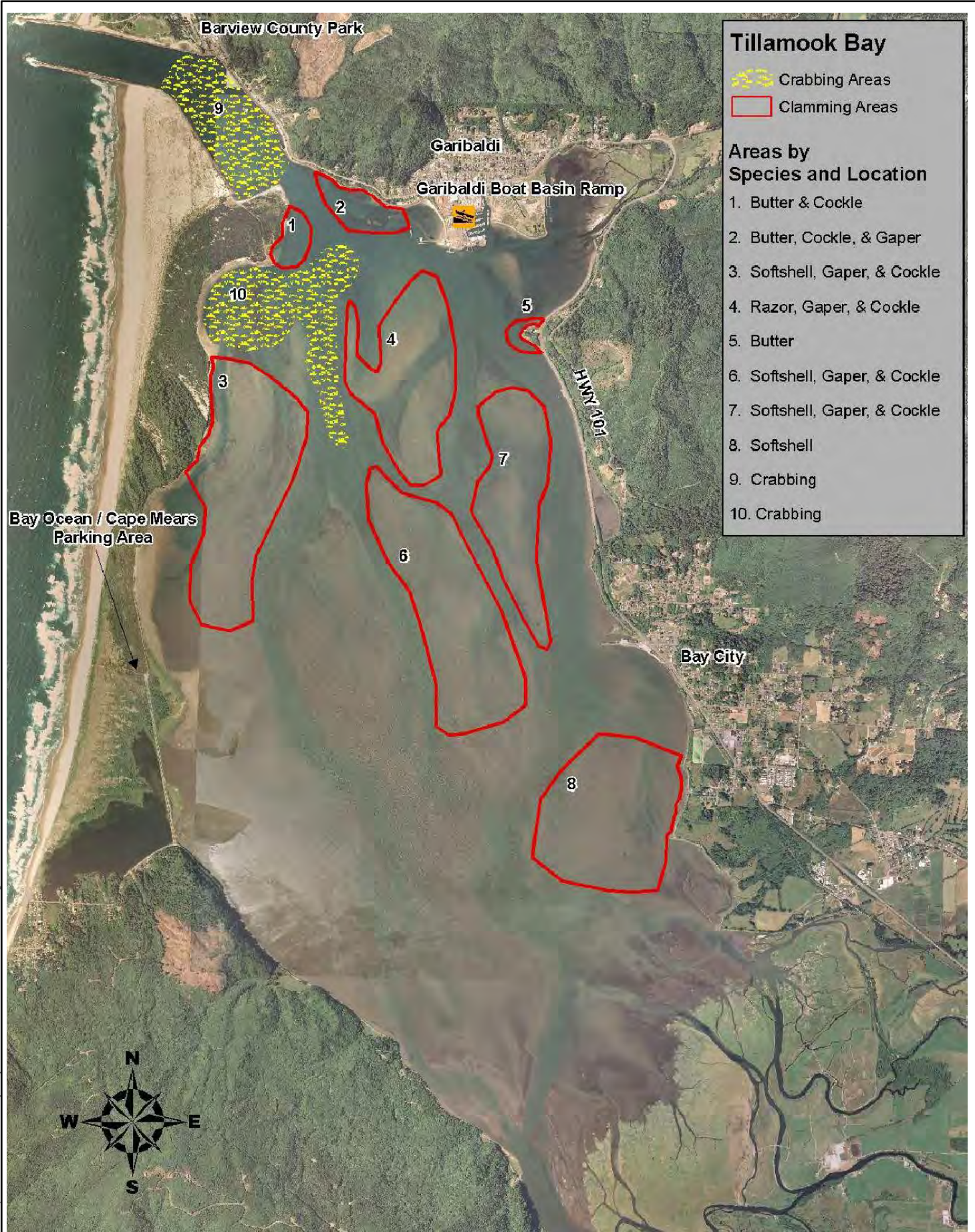
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Tillamook Bay

 Crabbing Areas

 Clamming Areas

Areas by Species and Location

1. Butter & Cockle
2. Butter, Cockle, & Gaper
3. Softshell, Gaper, & Cockle
4. Razor, Gaper, & Cockle
5. Butter
6. Softshell, Gaper, & Cockle
7. Softshell, Gaper, & Cockle
8. Softshell
9. Crabbing
10. Crabbing

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outfall diffuser in a deeper, more stable channel within the Bay. Historical NOAA navigation charts indicate this channel has been present at this location and has maintained mean low water depths of seven to nine feet for at least the past 90 years (1926 chart attached). Selection of the proposed outfall site considered the following:

- Water Depth
- Outgoing Tidal Currents
- Channel Stability
- Proximity to existing wastewater facilities
- Distance from designated shellfish reserve areas

Based on these criteria, the outfall site proposed will be located at Latitude N. 45.5237° and Longitude - 123.9005°.

Plan and profile views of the proposed outfall are presented in Figure 5.

4.0 Background Water Quality Data

Background water quality data for Tillamook Bay and the Bay City wastewater treatment plant effluent are discussed in the following section. The data are utilized to model ambient conditions assuming the worst case (1 in 10 year flow and treatment performance) discharge scenario.

4.1 Ambient Water Quality Data

The majority of information derived for the study of outfall impact on Tillamook Bay was obtained from various published studies and papers, which have been previously performed in association with some aspect of the Bay or its associated water shed.

4.1.1 Salinity and Temperature

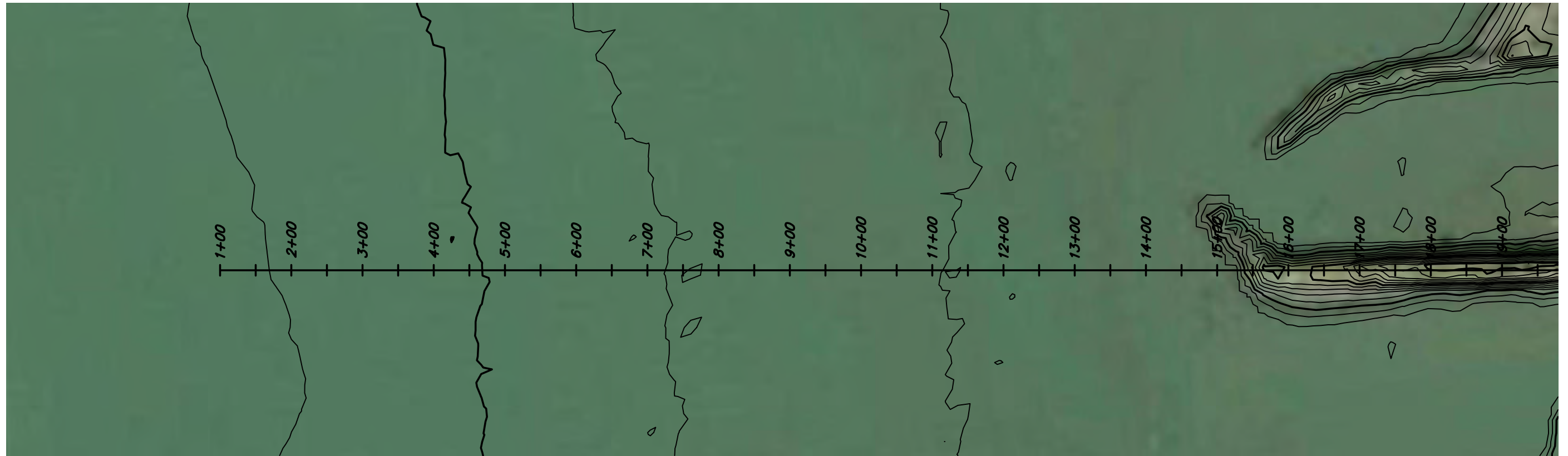
Past studies of the Bay have classified the estuary as a two-layered stratified system during the high run-off periods (November through March) and well-mixed, vertically homogeneous during low flow periods (April through October). The studies suggest that due to the large tidal amplitude, shallow depth and moderate freshwater inflow, stratification is not sustained for extended periods of time.

Salinity and temperature conditions were derived from two previous studies associated with Tillamook Bay. Overall, it appears that salinity and temperature conditions were similar between the two studies. In both studies, mean temperatures in the mid-region of the Bay varied over a wider range than in the lower Bay due to shallow depths and the strong influence of air temperature. Mean temperature associated with measurements taken in April through July, in the midregion of the Bay was 14.4° C and ranged from 9 to 17° C, dependent upon the season.

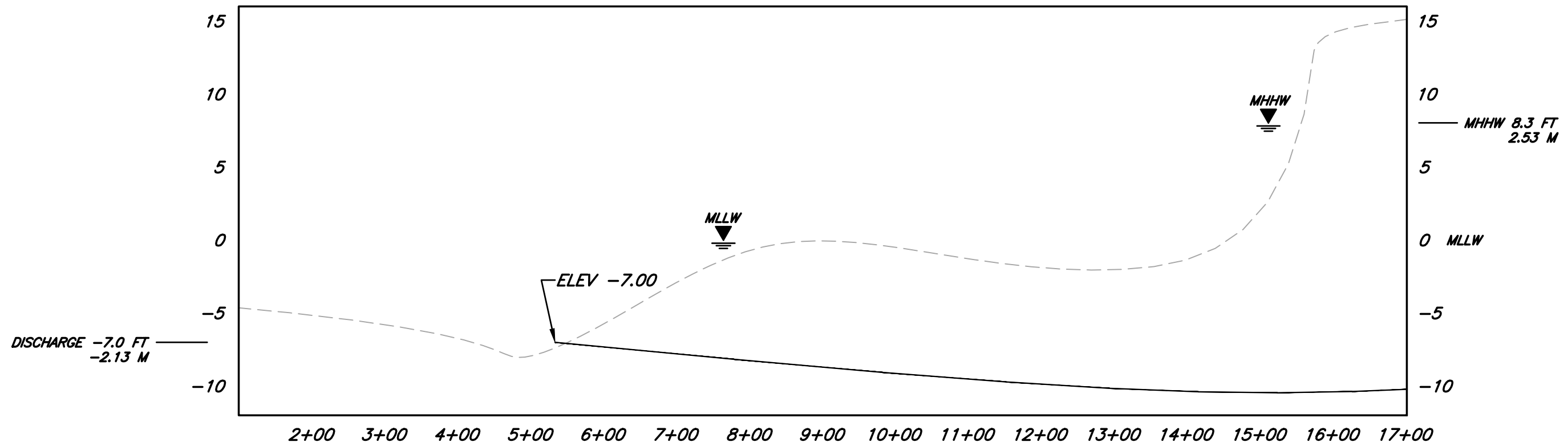
Mean salinity associated with measurements taken in April through July, in the midregion of the Bay was 22.8 ppt and ranged from 18.3 to 31 ppt, dependent upon the season. In comparison, the Pacific Ocean has an average salinity of 35 ppt.

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PLAN
1"=20'



PROFILE
SCALE: 1"=150' H
1"=2' V



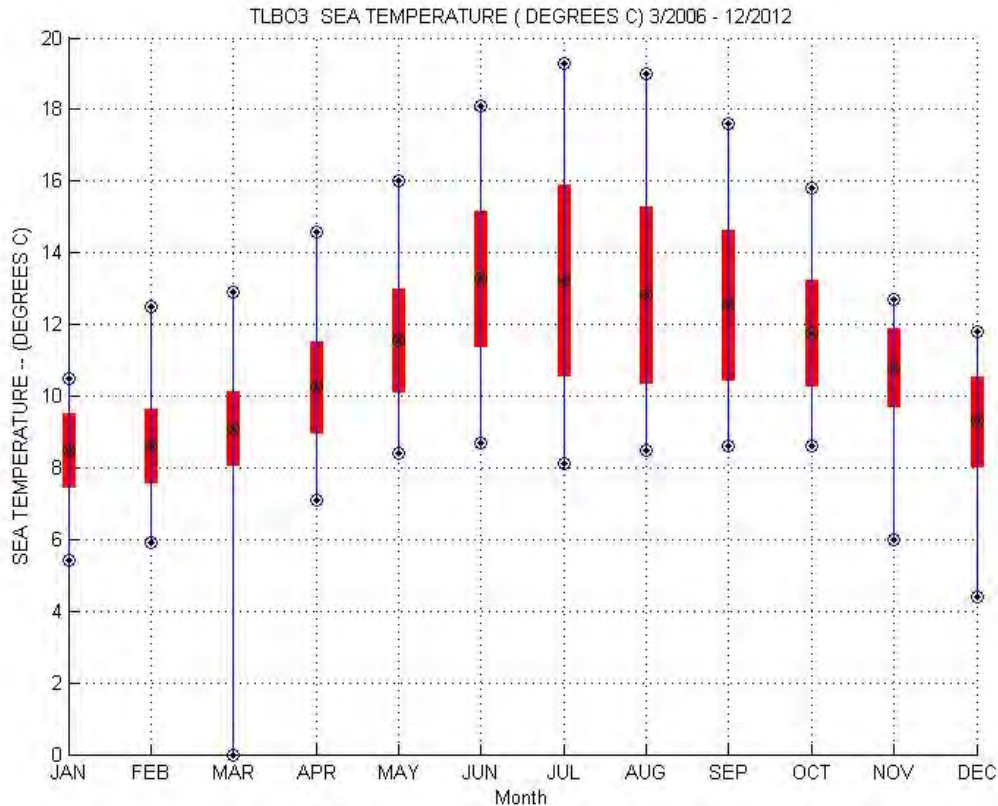
CITY OF BAY CITY
WASTEWATER FACILITIES PLAN
Bay City, Oregon
January 2010

PROPOSED WW OUTFALL
PLAN/PROFILE
SHN 010000
611013-OFALL
Figure 5

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The following graph, downloaded from NOAA, National Buoy Data Center, for readings made at the Garibaldi station (located on the Bay approximately 12,000 ft north of the proposed outfall sit), depicts a recent history of lower bay temperatures:

Figure 6
Station TLBO3 - Climatic Summary Plots for Sea Temperature
Mean and Standard Deviation Plot



4.1.2 Water Depth

Water depth at the proposed outfall location is affected by tidal forces, storm surges and hydrologic inputs from the upstream drainage basins. For modeling purposes, the average tidal forces will be assumed to control water levels elevations. The National Oceanographic and Atmospheric Ocean Administration collect data on water elevations. The nearest tidal data station closest to the outfall site is the Garibaldi Station (9437540). The elevations for this station are as follows:

Highest Observed Water (12/31/2005)	15.91 feet
Mean Higher High Water	12.3 feet
Mean High Water	11.59 feet
Station Datum (NAVD88)	0.00 feet
Mean Tide Level (MTL)	8.47 feet
Mean Low Water	5.35 feet
Mean Lower Low Water	3.99 feet
Lowest Observed Water (11/26/2007)	-0.52 feet

Under Mean Lower Low Water elevation, the estuary floor at the proposed outfall site will have 9 feet of water above it. In comparison, the existing disposal site will be completely exposed mud flats.

4.1.2 Density

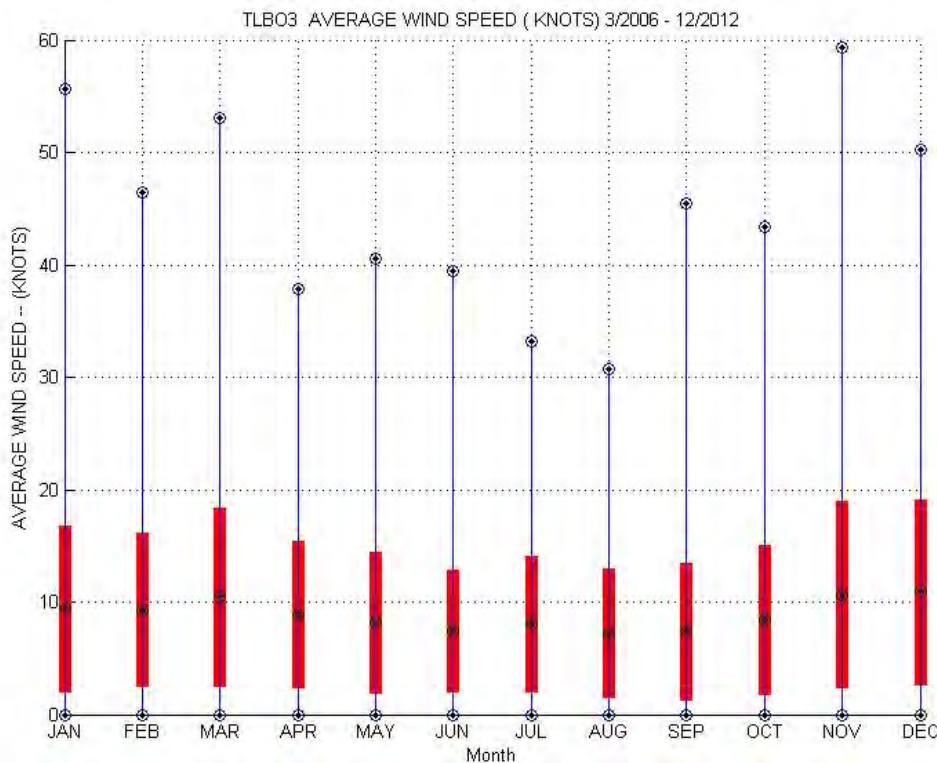
Density profiles for the outfall site were derived by correlating the density of seawater to the associated temperature and salinity. The range in density from 1,013 kg/m³ to 1252 kg/m³, depending on salinity and temperature. An average density of 1,136 kg/m³ will also be used for modeling purposes based on an average temperature of 14.4°C and a salinity of 22.8 ppt.

4.1.3 Winds

Mixing zone modeling will also consider the influence of wind speed on mixing once the discharge floats to the surface. Winds along the Tillamook estuary can be characterized as strong with predominate directions from either the northwest in the winter and southwest or southeast in the summer. Average wind speeds for the Garibaldi weather station, located on the north shores of the Tillamook Bay are shown in Figure 7 below. Based on the data provided from this weather station, wind speeds for modeling purposes will be based on an 11-knot wind (5.7 m/s).

Figure 7
Average Wind Speed at Garibaldi, OR

Station TLBO3 - Climatic Summary Plots for wind speed
Mean and Standard Deviation Plot



The strong winds occurring along the Estuary will generally enhance mixing at the surface layer but may also tend to push the effluent shoreward. Modeling of the diffuser structure should target identifying a design that creates a well-mixed plume at the surface layer to reduce the significance and risk of the strong winds pushing the effluent shoreward

4.1.4 pH

Data for water pH in Tillamook Bay is limited. The water quality data collected with the 1998 fish sampling in the Bay indicates a pH of 9.92 in the vicinity of the proposed outfall site, taken on July 15, 1998. Considering the tidal influence in the mid Bay location of the proposed outfall site, the pH of the ocean will be used for data representing the lower values of the Bay.

The ocean pH is maintained by the net charge balance of positive ions (Na⁺, K⁺, Mg⁺⁺, and Ca⁺⁺) and negative ions (Cl⁻, SO₄⁻, and Br⁻). Whenever there is a slight charge imbalance, the ocean buffering capacity neutralizes the additional acid or base. This buffering ability is controlled by the carbonate system that balances the ocean pH in a narrow range between 7.6 and 8.1.

4.1.5 Summary of Ambient Properties

Ambient properties proposed as representative of the Tillamook Bay at the outfall site are provided in Table 3 below.

Table 3 Summary of Tillamook Bay Ambient Properties

Ambient Property	Lowest	Average	Highest
Water Temperature	11.8°C	14.4°C	17°C
Density, kg/m ³	1,013	1,136	1,252
Density Gradient, kg/m ³	none	none	none
pH	7.6	8.2	8.9
Ammonia as N µg/L	0.10	1.0	3.4
Wind speed, knots	0	11	60

4.2 Effluent Quality Characteristics

Effluent characteristics for parameters of concern will be controlled by the efficiency of the wastewater treatment plant. Based on the NPDES permit, the discharge of secondary effluent should meet a 30 mg/L BOD and 30 mg/L TSS criteria. A summary of the NPDES effluent limitations for the WWTP is provided in Table 4 below.

Table 4 NPDES Permit Limits

May 15 – Oct 30	
BOD	30
TSS	30
Nov 1 – May 14	
BOD	30
TSS	30

Table 4, Continued NPDES Permit Limits

Year-round	Year-round Limitations
<i>E. coli</i> bacteria:	Shall not exceed 126 organisms per 100 ml monthly geometric mean. No single sample shall exceed 406 organisms per 100 ml.
pH:	Shall be within the range of 6.0 to 9.0
BOD ₅ and TSS Removal Efficiency:	Shall not be less than 85% monthly average based on ADWF and AWWF for the facility.
Total Chlorine Residual:	No chlorine or chlorine products shall be allowed.
* Based on average dry weather flow (ADWF) to the facility of 0.132 mgd and average wet weather flow (AWWF) of 0.248 mgd.	

4.2.1 Ammonia

The ammonia concentrations in the discharge is not an NPDES parameter of concern. However, ammonia testing has been performed on plant effluent once a week since 2009. Effluent ammonia ranged from 0.0016 mg/L to 13.93 mg/L, with an average of 1.37 mg/L over the past 8 years.

Variations in effluent ammonia will depend on operational modes of the Sequencing Batch Reactor (SBR) process. During the summer, the treatment plant will be operated to promote nitrification resulting in lower effluent NH₃-N concentrations. Average summer time ammonia concentrations over the past 8 years have been 1.77 mg/L.

During the winter, higher flows may require the plant to operate in a high rate batching process that does not allow nitrification. It is reasonable to expect ammonia removal to be limited by the rapid process time and lower temperatures. Average winter time ammonia concentrations over the past 8 years has been 1.09 mg/L.

The 90th percentile of exceedence (1 in 10 year) ammonia level over the past 8 years is 6.79 mg/L and will be used as the upper limit of ammonia concentration in the discharge for modeling purposes

Ammonia toxicity criteria that determine mixing requirements are defined in Table 2. Worst case scenario for toxicity criteria are encountered with ambient properties of 17 °C, a pH of 8.9 and salinity of 18.2 ppt. These conditions represent ambient conditions in the highest range of temperature and pH along with the lowest range of salinity found in previous documentation of estuary conditions.

4.2.2 Effluent Temperature

Wastewater temperature is known to vary seasonally and generally parallels climatic temperatures. Data evaluated from the Bay City Wastewater Treatment Plant Discharge Monitoring Report (DMR) for effluent temperature suggests winter temperature ranges between 7 °C and 21 °C and during the summer ranging between 11 °C and 21 °C. The maximum thermal discharge that could influence the Bay temperature for the winter and summer seasonal average is 21 °C. Temperature standards for the protection of fish relate to the mid bay region, that is associated with rearing and migration is a maximum of 17.8°C.

4.3 Constituents of Concern

Summaries of effluent properties of concern are provided in Table 5 below.

Table 5 Summary of Effluent Properties and Required Standards

Ambient Property	Lowest	Average	Highest	Standards
Biochemical Oxygen Demand, mg/L	1	2.9	12	30
Total Suspended Solids, mg/L ¹	1	6.3	22	30
<i>E coli</i> bacteria, #/100ml ²	2	6.7	38	End of Pipe
Summer Temperature, °C	11	16.5	21	17.5
Winter Temperature, °C	7	13.1	21	
Density, kg/m ³	998	998.8	1000	N/A
pH	6.0	6.9	7.3	6-9
Summer Ammonia as N, mg/L	0.004	1.77	13.93	1.04 CMC ⁴ Acute
Winter Ammonia as N, mg/L ⁵	0.0016	1.09	8.80	0.16 CCC ⁶ Chronic
1. mg/L – milligrams per liter 2. ml - millileter 3. kg/m – kilograms per meter 4. CMC – Criteria Maximum Concentration 5. Wastewater Treatment Plant Design, WPCF MOP 8 6. CCC – Criteria Chronic Concentration				

4.4 Hydraulic Loadings

The effluent discharge rate can have a significant impact on the receiving water because the loading of pollutants is based on the discharge rate but also because the comparison to receiving water velocities (estimated flow rate for ambient with assumed boundary conditions) and the effluent momentum affect the mixing phenomenon. Effluent will be discharged from the City's Wastewater Treatment Plant through the proposed installation of a new discharge pumping system. Pump discharge rates are dictated by the process of the existing Sequential Batch Reactor (SBR) treatment system used by the City. The SBRs are typically operated in 6-hour cycles producing a total of eight batches per day. However, during high flows, cycle times can be decreased to as low as 3-hour intervals.

Treated supernatant is withdrawn from the SBR basins utilizing a floating 10-inch diameter decanter mechanism and discharged for disinfection. Effluent from the SBR is disinfected using a single-channel with two UV disinfection systems. The UV system is designed to disinfect a peak decant flow rate of 2.8 MGD, assuming a transmissivity of 65%.

After UV disinfection, treated effluent will be discharged via effluent pump station to Tillamook Bay through the proposed 12 inch discharge pipeline and 8 inch discharge port outfall. The effluent pump station will be sized to accommodate the SBR decant rate of approximately 1,700 gallons per minute.

4.5 Discharge Velocities

Effluent discharge velocities will be controlled by the design of the diffuser port. According to the CORMIX model design recommendations, effluent discharge velocities should be high, ranging between 3 m/s to 8 m/s. Lower velocities may be allowed, but discharge velocities less than 0.5 m/s are not recommended.

It is important to note that the discharge port size will affect the analysis of the toxic dilution zone (by reducing the discharge length scale) described in the regulatory review section of this report. In general, smaller discharge ports will decrease the length scale, which in turn reduces the ZID. In some cases, it could become necessary to reduce the length of the mixing zone and install more, smaller area, diffusers to achieve toxic dilution criteria.

5.0 Effluent Modeling

A variety of models exist for evaluating effluent dilution. The primary mixing zone models currently in common use include simple dilution equations, DYNTOX, CORMIX, UM, RSB, UDKHDEN, PDS, and PDSM. The structure, applicability, assumptions, and complexity of each model are summarized in Table 6 below.

Table 6 Summary of Available Mixing Zone Model

Model	Discharge Depth	Receiving Water Depth	# Ports	Dimensions	Complexity
Jet-Momentum Eq.	submerged	any	single	one	low
River Initial Mixing Eq.	any	shallow	single	one	low
Ambient Dilution Eq.	any	shallow	any	one	low
CYNTOX	any	shallow	single	one	moderate
CORMIX	any	any	any	three	moderate
UM	submerged	any	any	three	moderate
RSB	submerged	deep	multiple	three	moderate
PDS	surface	deep	single	three	high
PDSM	surface	deep	single	three	high
UDKHDEN	submerged	deep	multiple	three	high

Model selection for this study was based on the objectives of the study, availability and quality of input data, and the specific conditions of the perceived discharge scenario proposed to be modeled. The following selection criteria were applied to select the model used in this study:

- Input data was limited; therefore the model should not require extensive input data
- The results of the model should be reproducible by others, therefore the model should be readily available, well-documented, and relatively easy to use
- Schedule and budgetary limits were critical to the project, therefore model input parameters should be easy to adjust, enabling relatively rapid analysis of various outfall configurations
- The results of the study are subject to DEQ examination; therefore the model should be recognized and accepted by regulators

Based on these criteria, CORMIX was selected for use in this study. CORMIX satisfies the criteria listed above as follows:

- CORMIX is a length scale analytical model. As such, it does not require extensive input data, as might be required for multi-dimensional numerical models.
- CORMIX engine is a public domain model; readily available and well documented.
- The CORMIX interface allows input parameters to be adjusted relatively quickly, permitting sensitivity analyses to be conducted expeditiously
- CORMIX has been accepted and endorsed by the USEPA and Oregon DEQ
- As an expert system, CORMIX provides design recommendations, which can enhance the design process and lead to an improved diffuser system

5.1 Mixing Zone Modeling Parameters

EPA's *Technical Support Document for Water Quality-based Toxics Control (TSD)* describes the critical design flows that should be used when performing mixing zone analyses for the various waterbodies. EPA's *TSD* defines estuaries as having a main channel reversing flow and coastal bays as having significant two-dimensional flow in the horizontal directions. For both water bodies, the critical design conditions recommended by EPA are based on a combination of the tides and the river conditions. Because plume dynamics within an estuarine environment are so complex, discharge dilution cannot be calculated simply based on the receiving stream critical low flow and the effluent discharge rate. Effluent mixing within an estuary is complicated by density stratification, tidal variation, wind effects, riverine inputs, and complex circulation patterns.

In addition to evaluation of the above critical design conditions, an off-design condition was evaluated as well. The recommended off-design condition for both stratified and unstratified conditions is that of maximum velocity during a tidal cycle. It was assumed the off-design condition would likely result in greater dilution but it could carry the plume further downstream. An evaluation of this condition was made to assure toxic conditions are not carried downstream into critical resource areas such as shellfish habitat.

For application of acute criteria, the 10th % velocity over one tidal cycle was used for critical slack conditions and 90th % for the off-design condition. For chronic and human health criteria the 50th% velocity was used.

5.1.1 Detailed Tidal Simulations

A high variation in both ambient velocity and tidal elevation occurs during the tidal episode shown in Figure 8. In such highly time-variant ambient conditions, CORMIX recommends predictions are performed at critical tidal conditions throughout a reversal episode. These critical tidal conditions are identified as:

1. Shortly after slack tide: Effects of re-entrainment of discharge from the previous half-cycle are greatest. However, the flow is evolving rapidly in time, causing CORMIX tidal predictions to be limited in spatial extent. Several predictions should be made at hourly or half hourly intervals following the reversal.
2. Maximum flood and ebb currents: These represent extremes of along-shore extent and shoreline interaction. Re-entrainment will be less important at these times.

Per CORMIX guidance, seven simulations were performed at the times indicated on Figure 8 by the letters a-g. In the following section, a detailed simulation is performed corresponding to time b, one hour after slack tide. The results are contrasted for that case to the steady-state assumption simulated in the preceding.

Along with the recommended CORMIX tidal simulations, four simulations were performed to correlate with the prescribed conditions by the EPA's TSD.

Figure 8 represents the ambient water conditions associated with each simulation.

Minimal initial dilution generally will not occur at slack tide, but shortly after slack tide when the plum re-entrains material remaining from the previous tidal cycle. In tidal mode, CORMIX considers the reduction in initial dilution due to reentrainment of material remaining from the previous cycle.

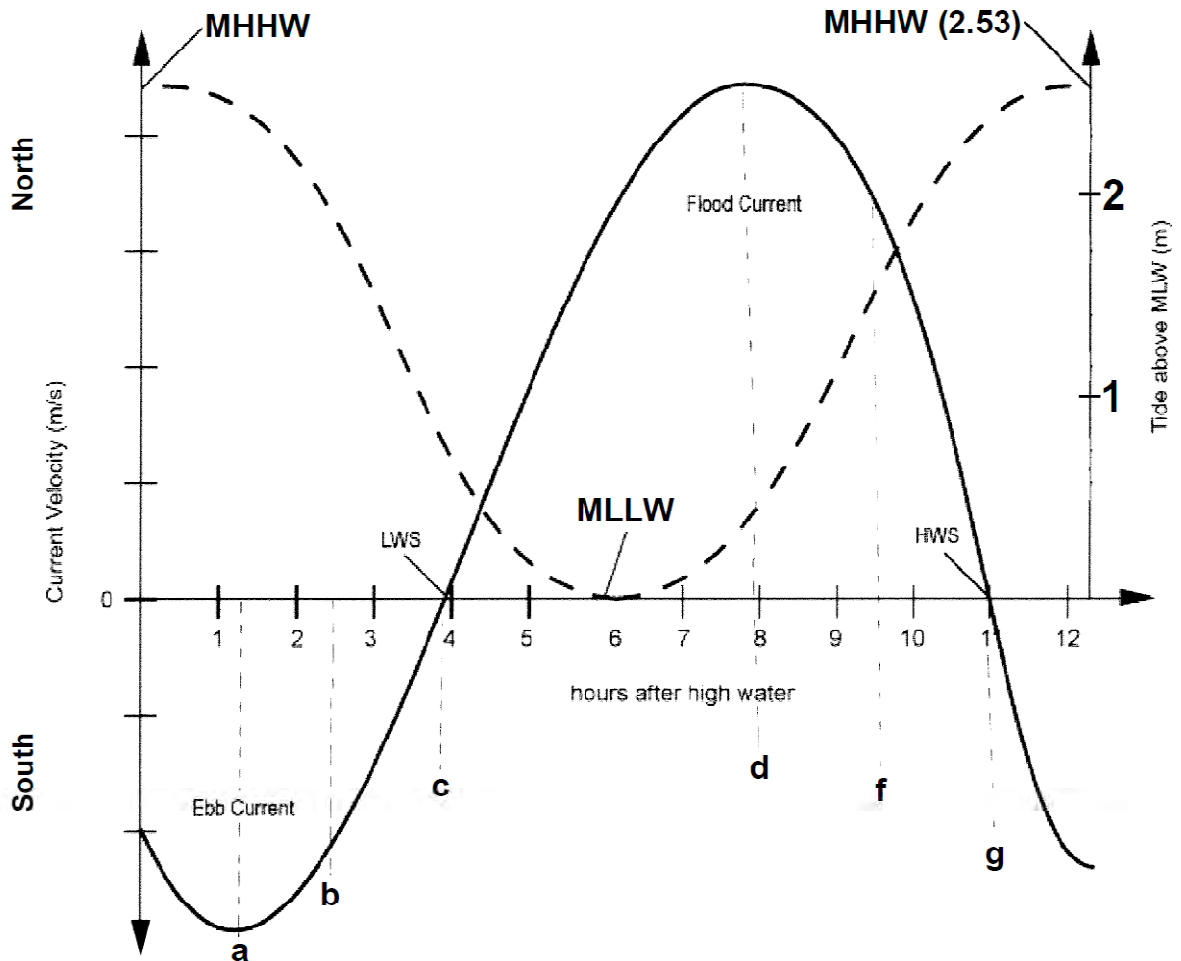


Figure 8.

Table 7 Detailed Tidal Simulations

Flow Scenario		Tidal Elevation (ft)	Tidal Elevation (m)	Depth to Bottom (m)	Average Depth (m)	Average Depth (ft)	Maximum Velocity (m/s)	Current Velocity (m/s)	distance to left bank (m)
Ebb Current	a	7.9	2.4	5.14	16.9	unbounded	0.75	unbounded	
	b	4.5	1.37	4.11	13.5	unbounded	0.5	unbounded	
	c	1.3	0.4	3.14	10.3	unbounded	0.01	unbounded	
Low Water Slack	d	0.0	0	2.74	9.0	560	0.8	unbounded	
	e	8.3	2.53	5.27	17.3	unbounded	1	unbounded	
	f	5.9	1.8	4.54	14.9	unbounded	0.75	unbounded	
High Water Slack	g	7.4	2.25	4.99	16.4	unbounded	0.01	unbounded	
	10th % velocity over one tidal cycle	0.7	0.2	2.94	9.7	unbounded	0.1	unbounded	
	90th % for off-design (impacts on shellfish habitat)	0.0	0	2.74	9.0	560	0.9	unbounded	
Acute Criteria	50% velocity (Ebb)	0.7	0.2	2.94	9.7	unbounded	0.5	unbounded	
	50% velocity (Flood)	6.6	2	4.74	15.6	unbounded	0.5	unbounded	



5.2 Mixing Zone Modeling Results

Several simulations were run with various discharge port sizes and at various directional configurations to determine the most effective combination for mixing. For simplicity and cost effectiveness, a mixing zone based upon a single port discharge was evaluated. An eight inch diameter port, discharging perpendicular to the tidal flow directions ($\Sigma = 90^\circ$) with an upward angle (Θ) of 10° , was found to provide adequate mixing for all discharge scenarios under consideration. The eight inch diameter port discharges the effluent at an initial velocity of approximately 3.3 meters/sec. Due to the buoyant nature of the discharge (lower density than ambient receiving stream) the plume will eventually make its way to the surface. However, with the proposed discharge velocities and the modeled scenarios, the Zone of Immediate Dilution is achieved prior to plume surfacing.

A summary of the results from mixing zone analyses in which the distance from the port where the CMC and CCC are met are provided in Table 8 presented below:

Table 8 Mixing Zone Analyses

Flow Scenario		CMC (ZID)	CCC
		Dist. (m)	Dist. (m)
a.	Ebb Current	2.37	25.4
b.		2.85	18.26
c.	Low Water Slack	6.93	
d.	Flood Current	2.35	19.63
e.		2.28	32.75
f.		2.38	25.36
g.	High Water Slack	6.64	
Acute Criteria	10th % velocity over one tidal cycle	5.86	
	90th % for off-design (impacts on shellfish habitat)	2.31	22.88
Chronic and human health criteria	50% velocity (Ebb)	2.81	14.3
	50% velocity (Flood)	2.84	19.05

5.3 Summary of Mixing Zone Modeling Results

Under all conditions, modeling predicts that the single-port diffuser will meet acute and chronic criteria and achieve all water quality objectives for pollutants of concern. The mixing zone length of 70 meters (30 m upstream and 40 m downstream from each of discharge nozzle) is shown by the model to provide adequate protection for water quality. Based on the limiting criterion of the discharge length scale, a minimum toxic dilution zone of 7 meters, each direction, should be permitted to achieve the CMC criteria.

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**DEQ Basin-Specific
Criteria**

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**DEQ Basin-Specific
Criteria**

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State of Oregon Department of Environmental Quality

Basin-Specific Criteria (North Coast)

340-041-0230

Beneficial Uses to Be Protected in the North Coast Basin

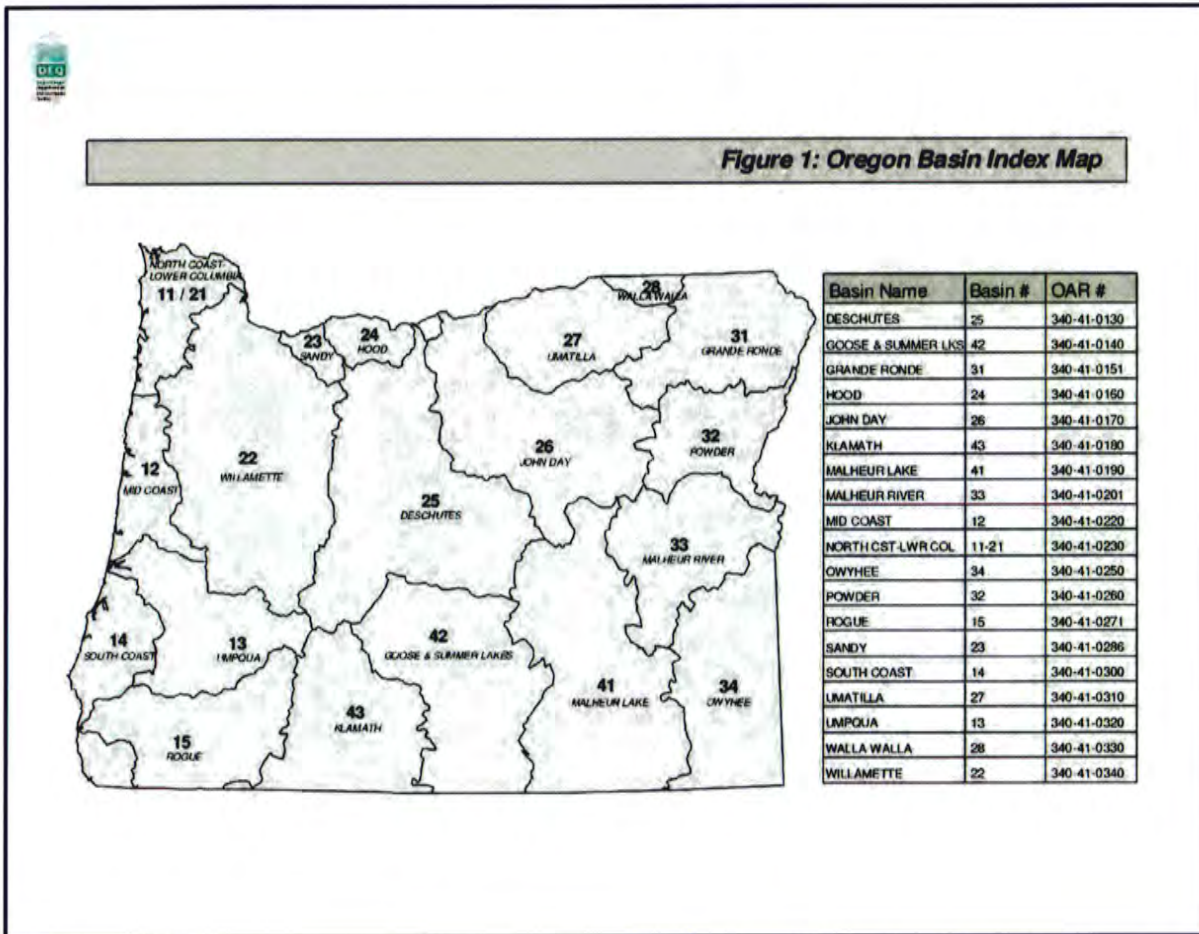




Table 230A
Designated Beneficial Uses
North Coast Basin
(OAR 340-041-0230)
(November 2003)

Beneficial Uses	Estuaries & Adjacent Marine Waters	All Streams & Tributaries Thereto
Public Domestic Water Supply ¹		X
Private Domestic Water Supply ¹		X
Industrial Water Supply	X	X
Irrigation		X
Livestock Watering		X
Fish & Aquatic Life ²	X	X
Wildlife & Hunting	X	X
Fishing ³	X	X
Boating	X	X
Water Contact Recreation ³	X	X
Aesthetic Quality	X	X
Hydro Power		
Commercial Navigation & Transportation	X	

¹ With adequate pretreatment (filtration & disinfection) and natural quality to meet drinking water standards.

² See also Figures 230A and 230B for fish use designations for this basin.

³ For coastal water contact recreation and shellfish harvesting uses, see also Figures 230C (Necanicum River Estuary), 230D (Nehalem Bay), 230E (Tillamook Bay), 230F (Netarts Bay), 230G (Sand Lake), and 230H (Nestucca Bay)

Figure 230A: Fish Use Designations*
North Coast Basin, Oregon

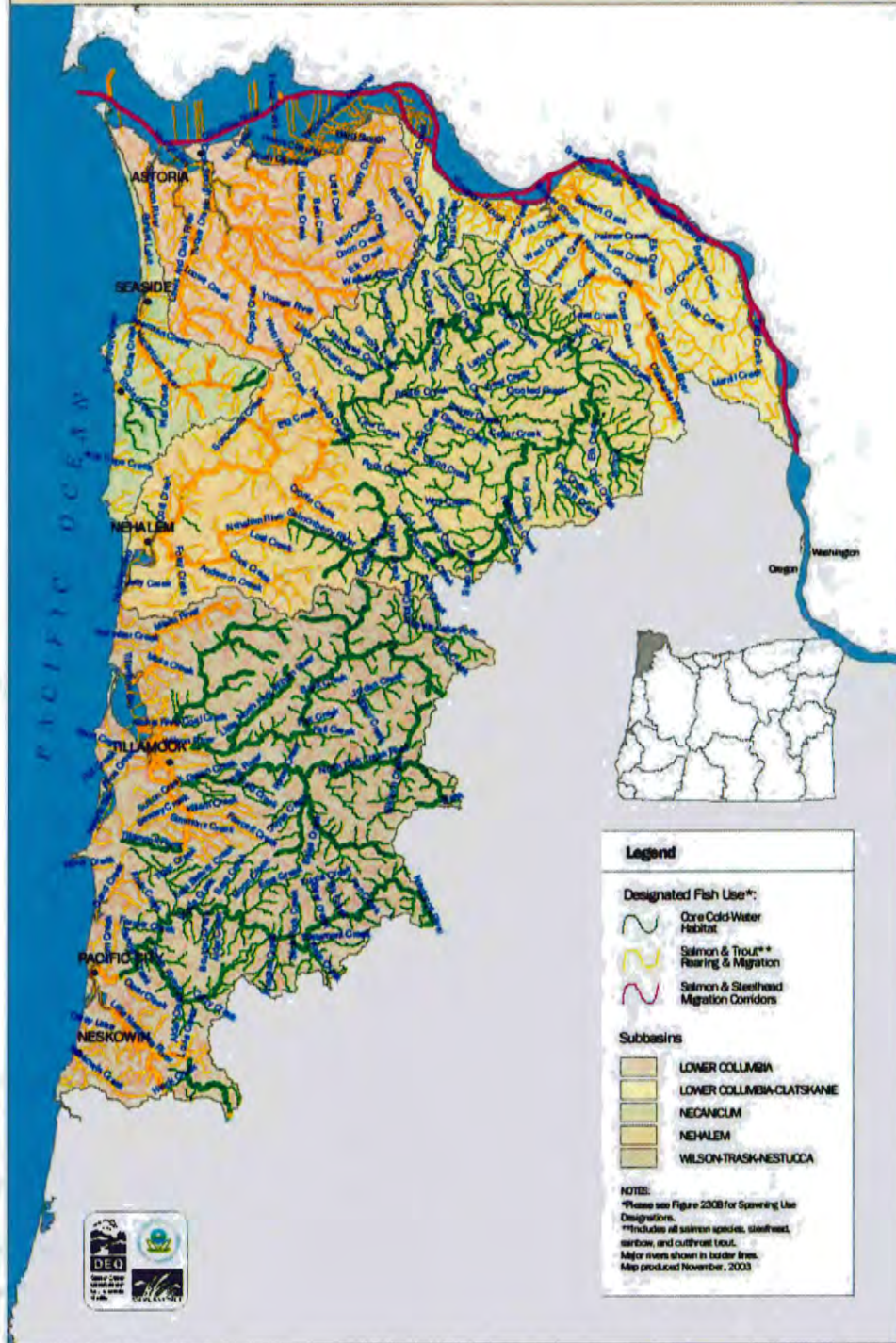


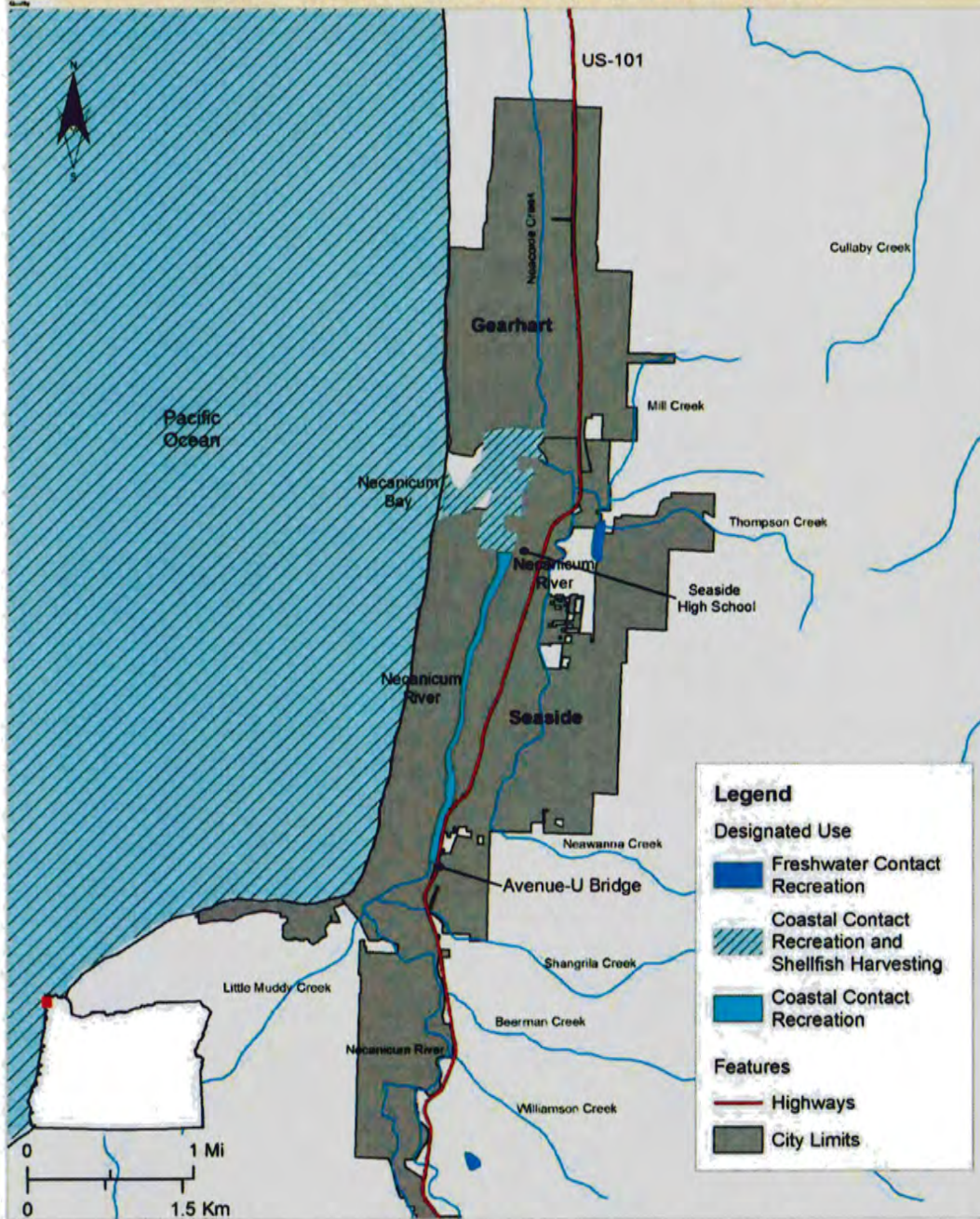
Figure 230B: Salmon and Steelhead Spawning Use Designations*
North Coast Basin, Oregon





OAR 340-041-0230

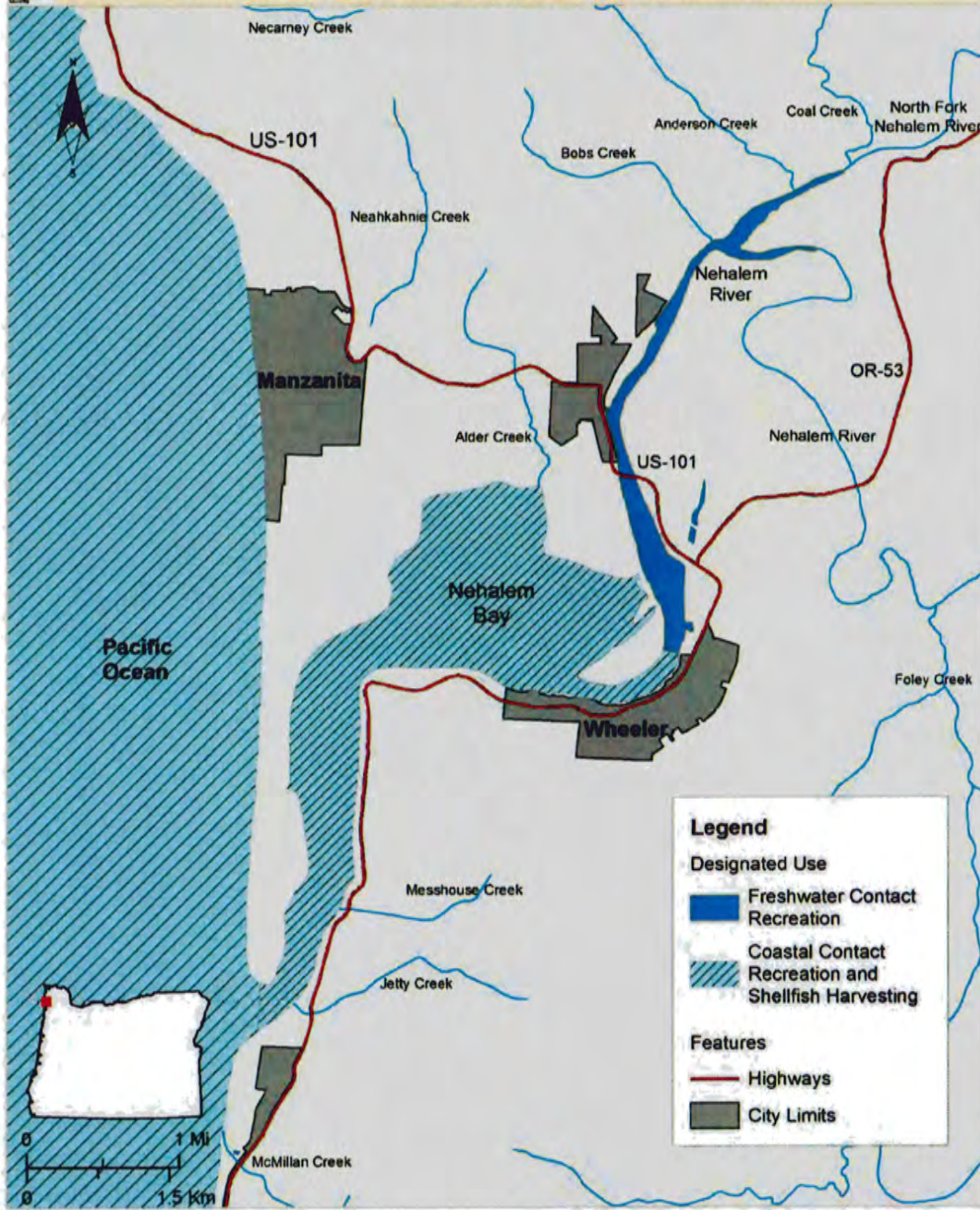
Figure 230C: Water Contact Recreation and Shellfish Harvesting Designated Uses
Necanicum Bay, North Coast Basin, Oregon (Draft February, 2016)





OAR 340-041-0230

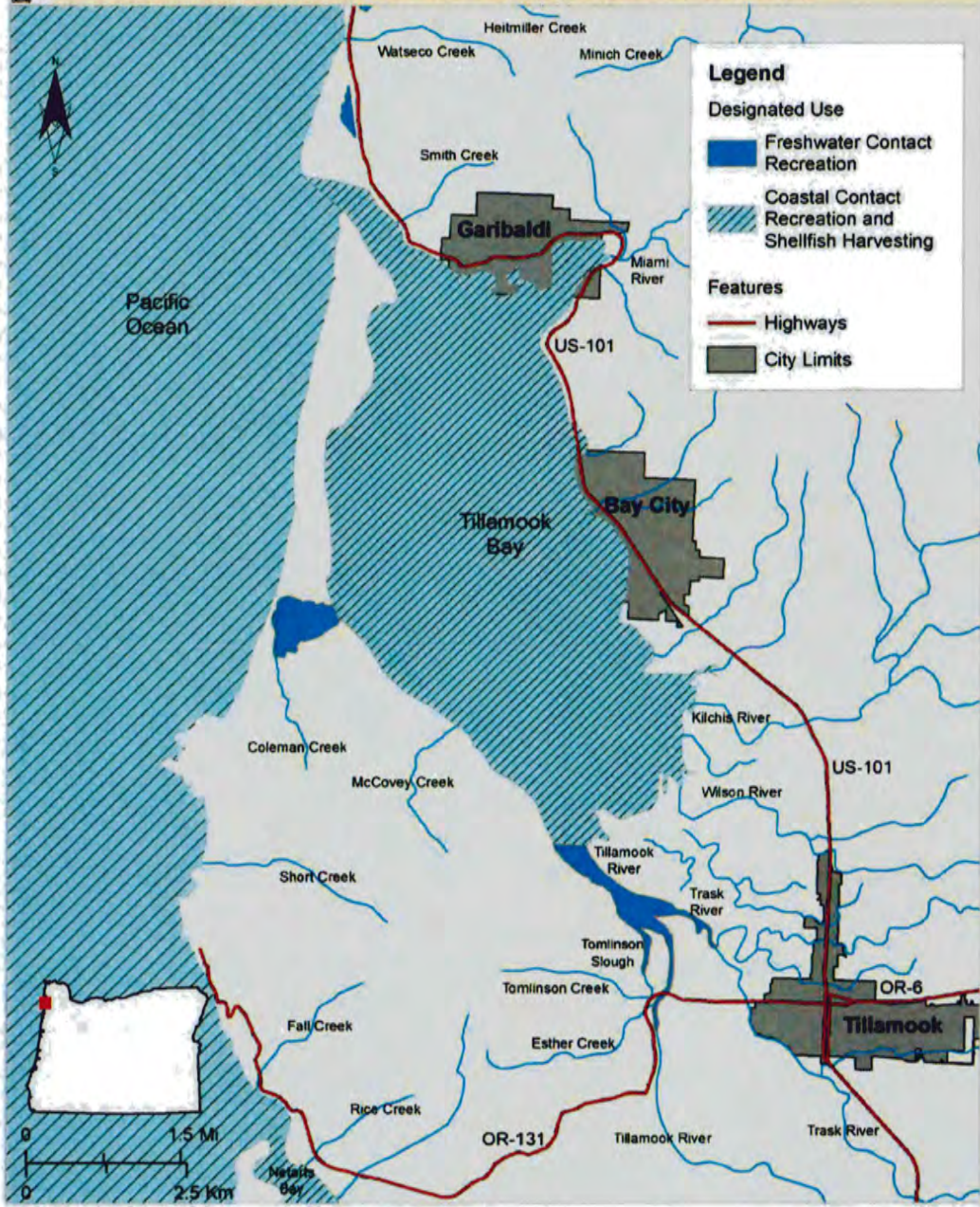
Figure 230D: Water Contact Recreation and Shellfish Harvesting Designated Uses
Nehalem Bay, North Coast Basin, Oregon (Draft February, 2016)





OAR 340-041-0230

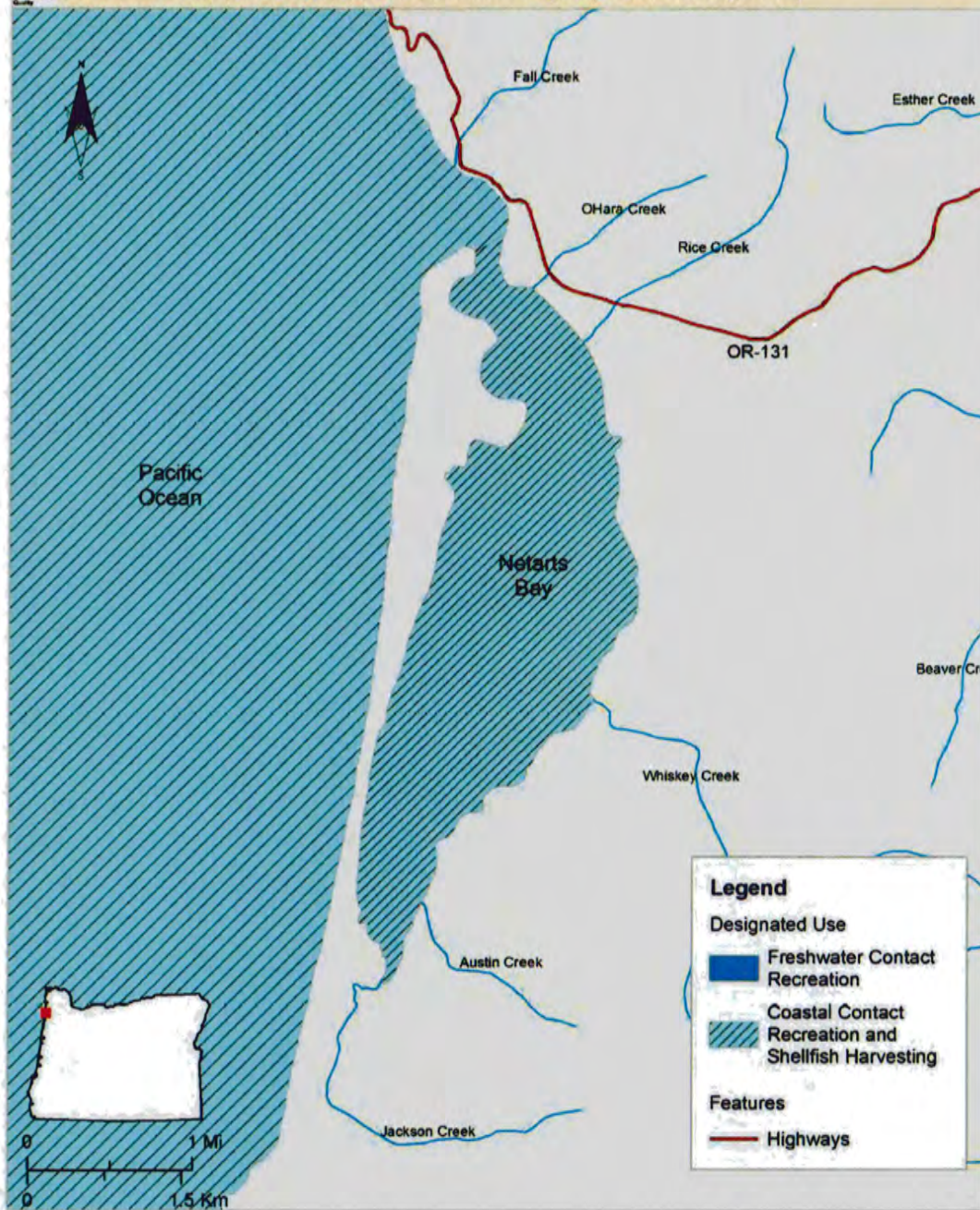
Figure 230E: Water Contact Recreation and Shellfish Harvesting Designated Uses
Tillamook Bay, North Coast Basin, Oregon (Draft February, 2016)





OAR 340-041-0230

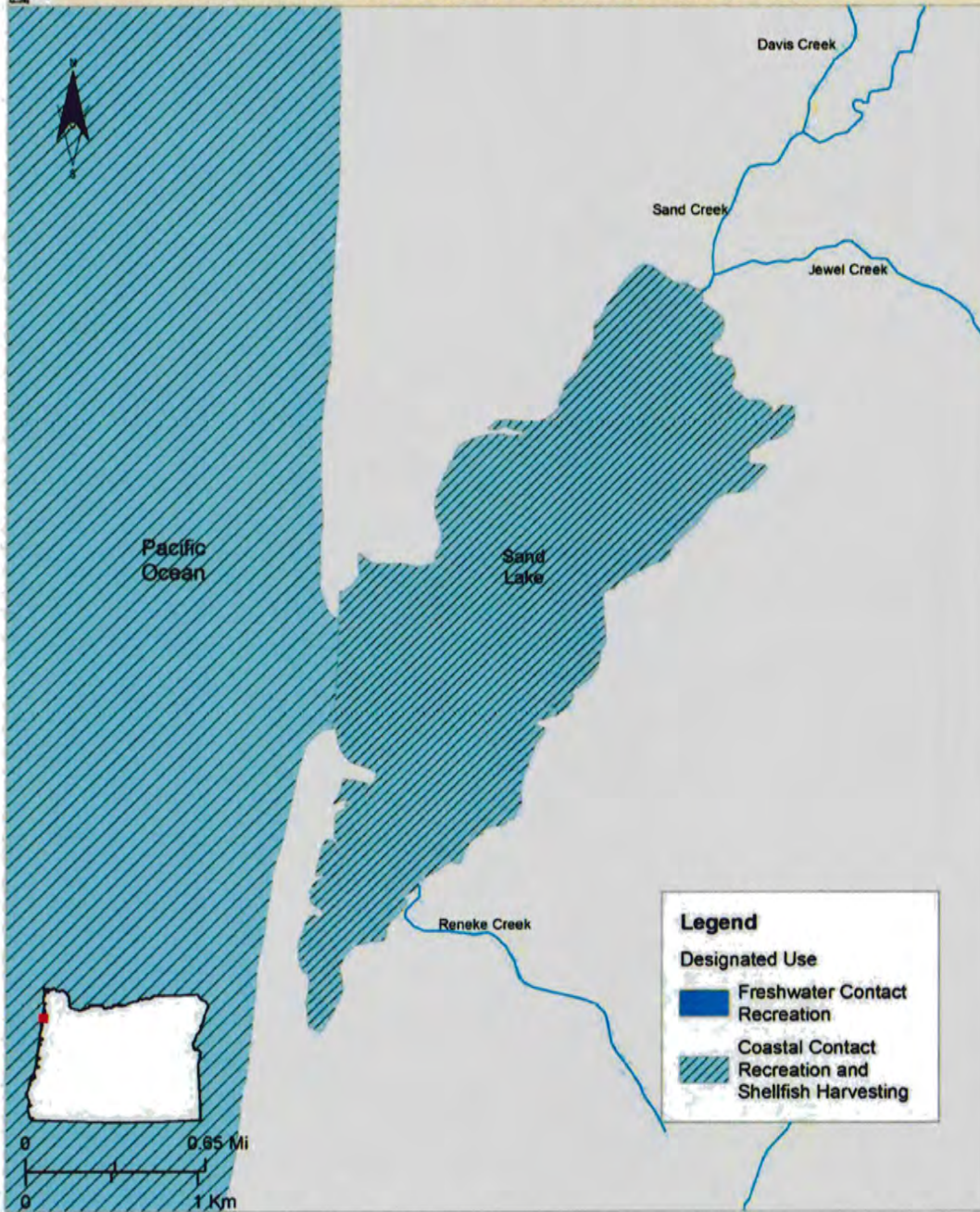
Figure 230F: Water Contact Recreation and Shellfish Harvesting Designated Uses
Netarts Bay, North Coast Basin, Oregon (Draft February, 2016)





OAR 340-041-0230

**Figure 230G: Water Contact Recreation and Shellfish Harvesting Designated Uses
Sand Lake, North Coast Basin, Oregon (Draft February, 2016)**



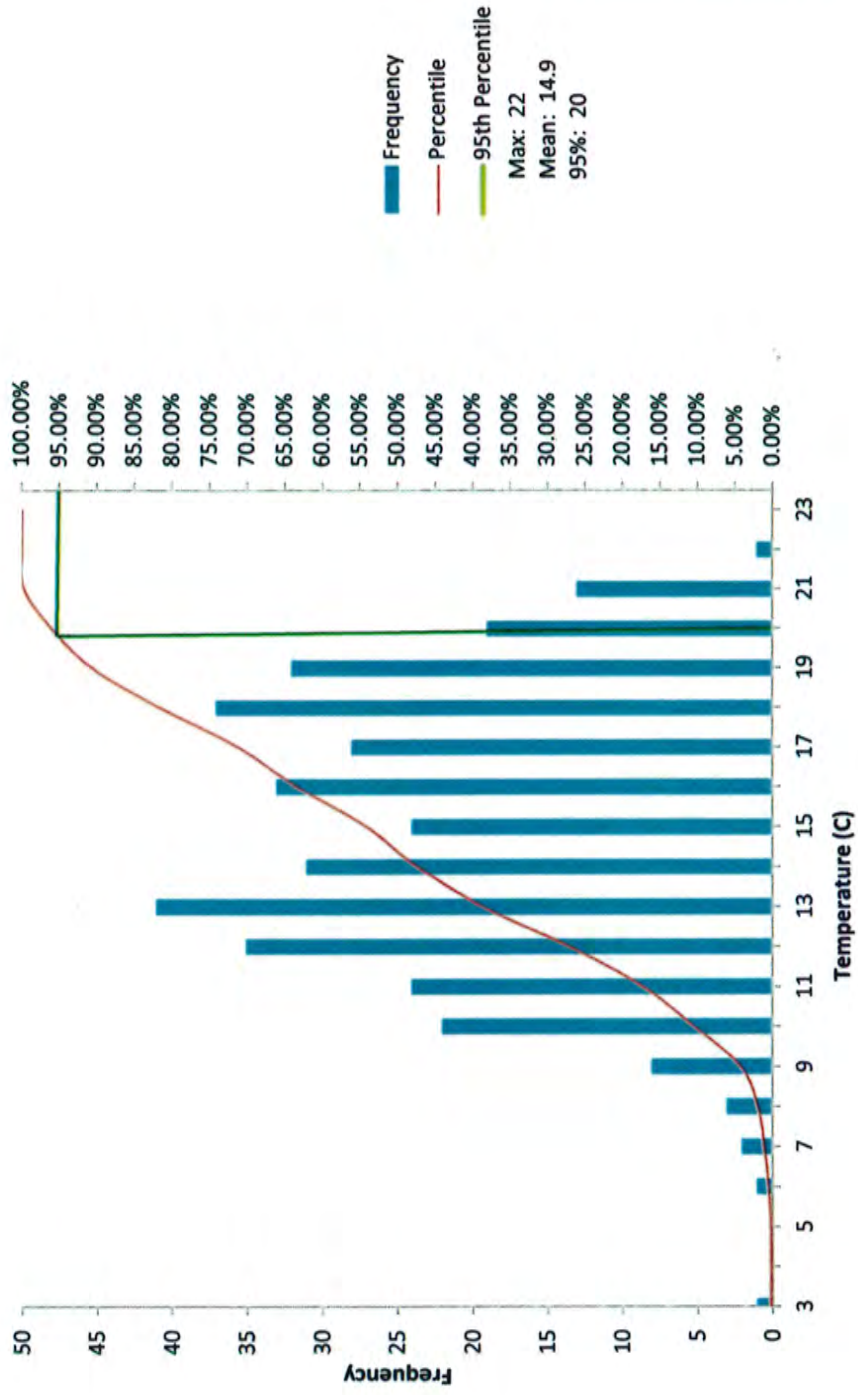
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Effluent Characteristics

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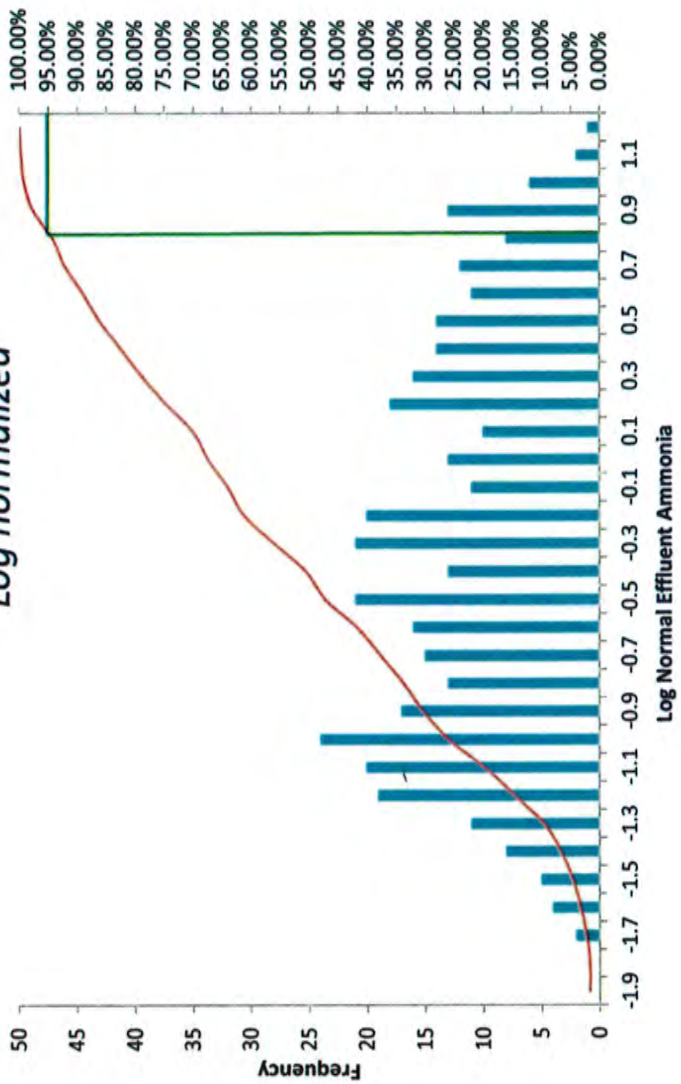
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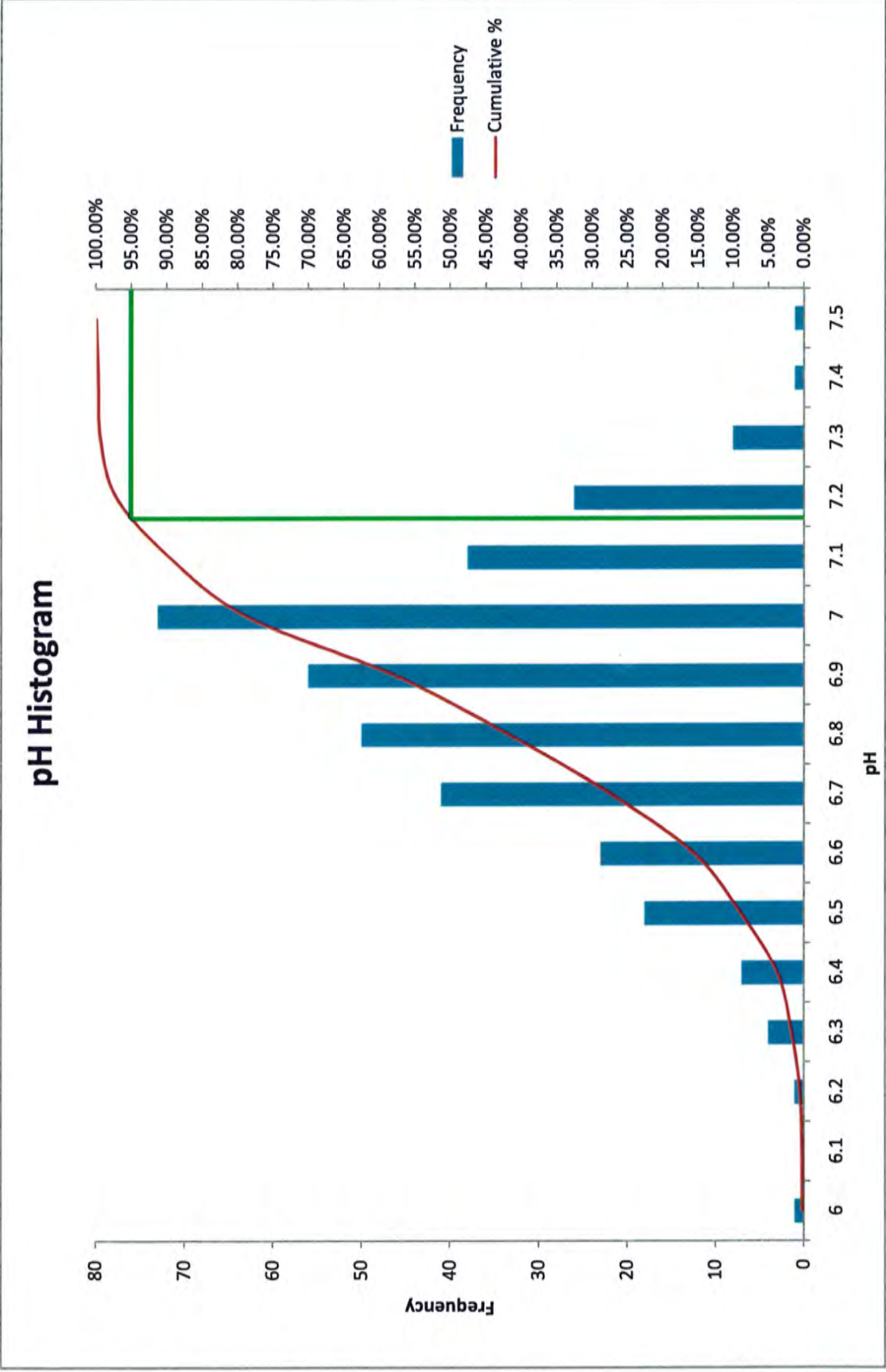
Temperature Frequency Histogram



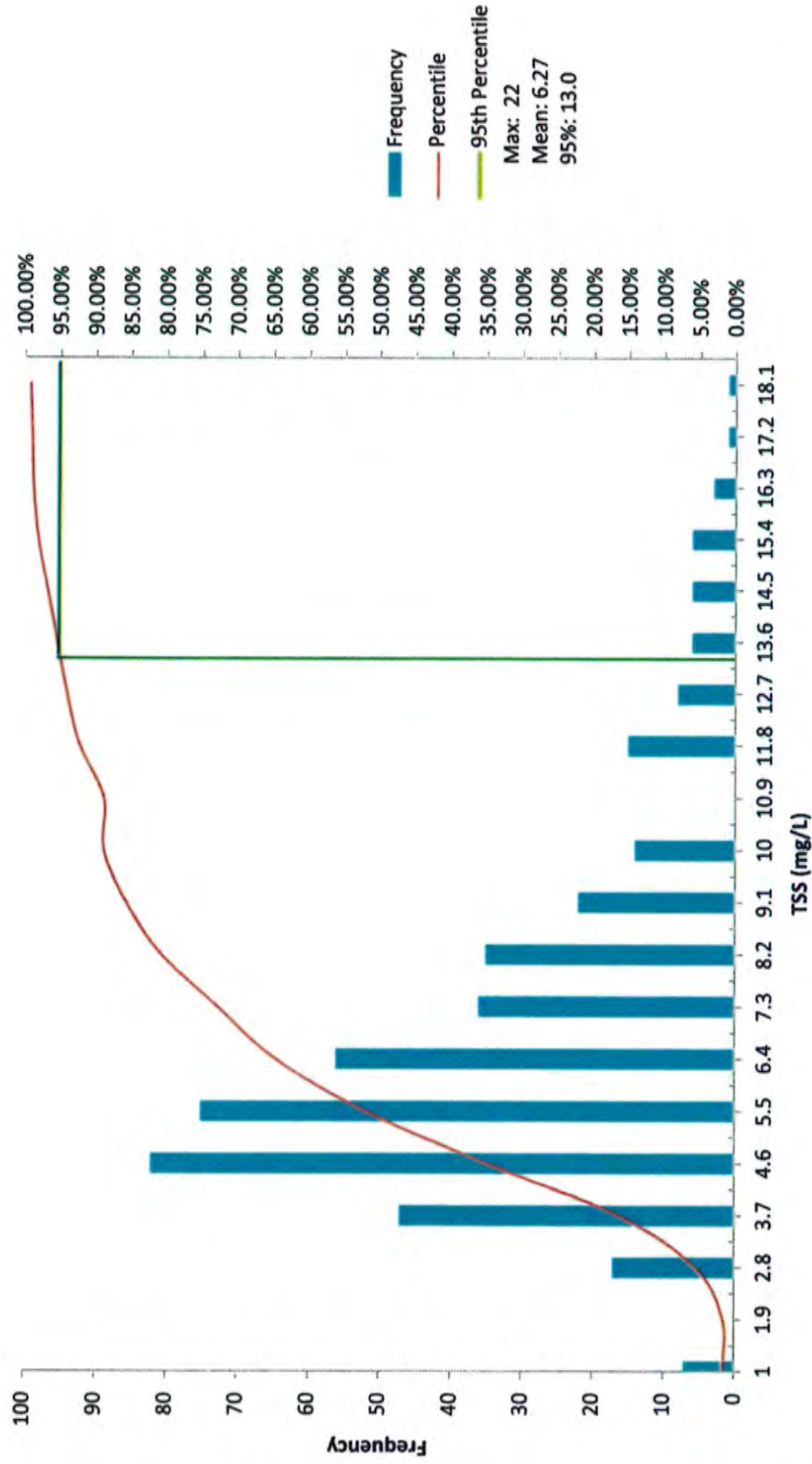
Frequency Histogram, Effluent Ammonia

Log normalized

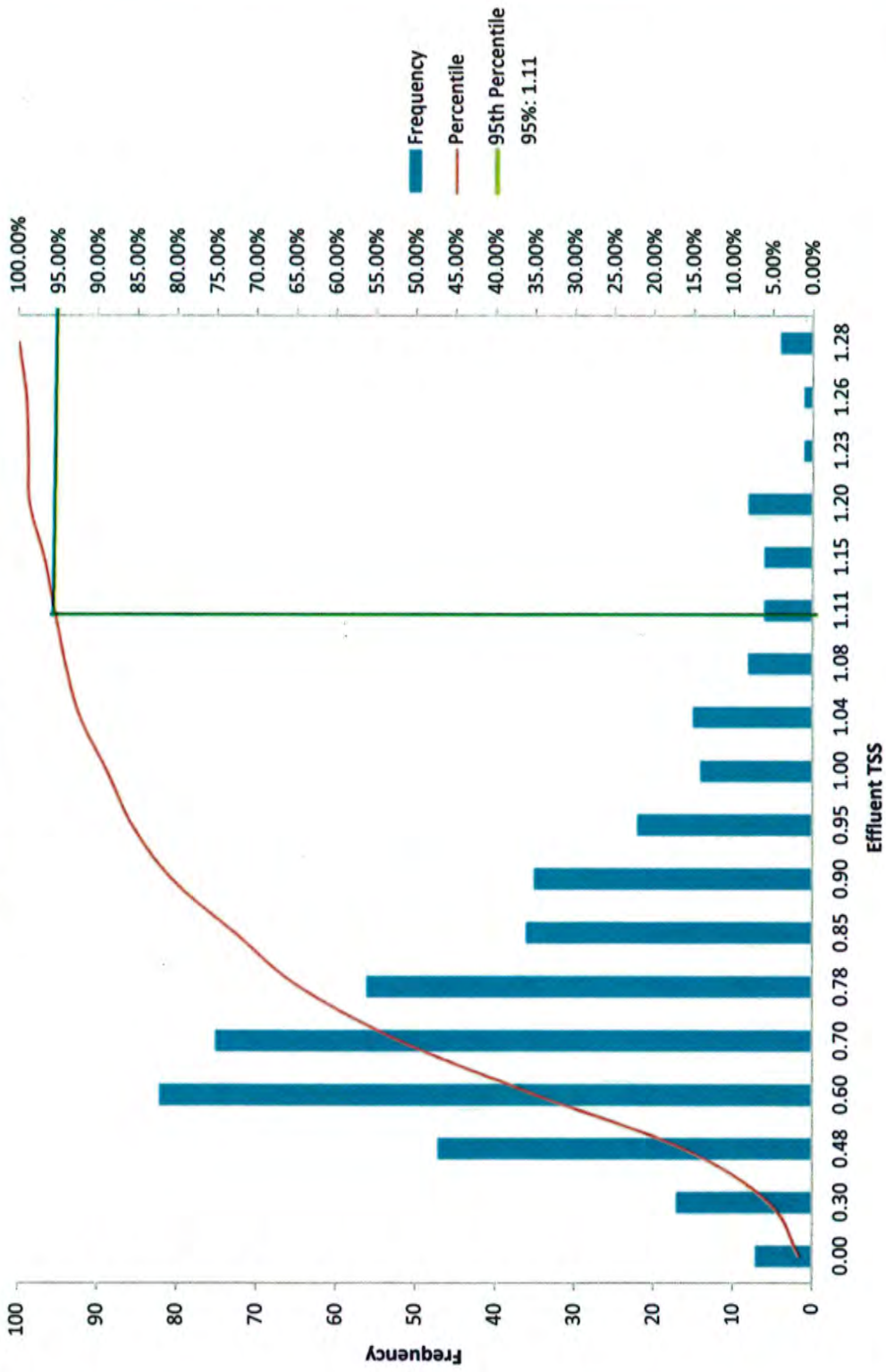




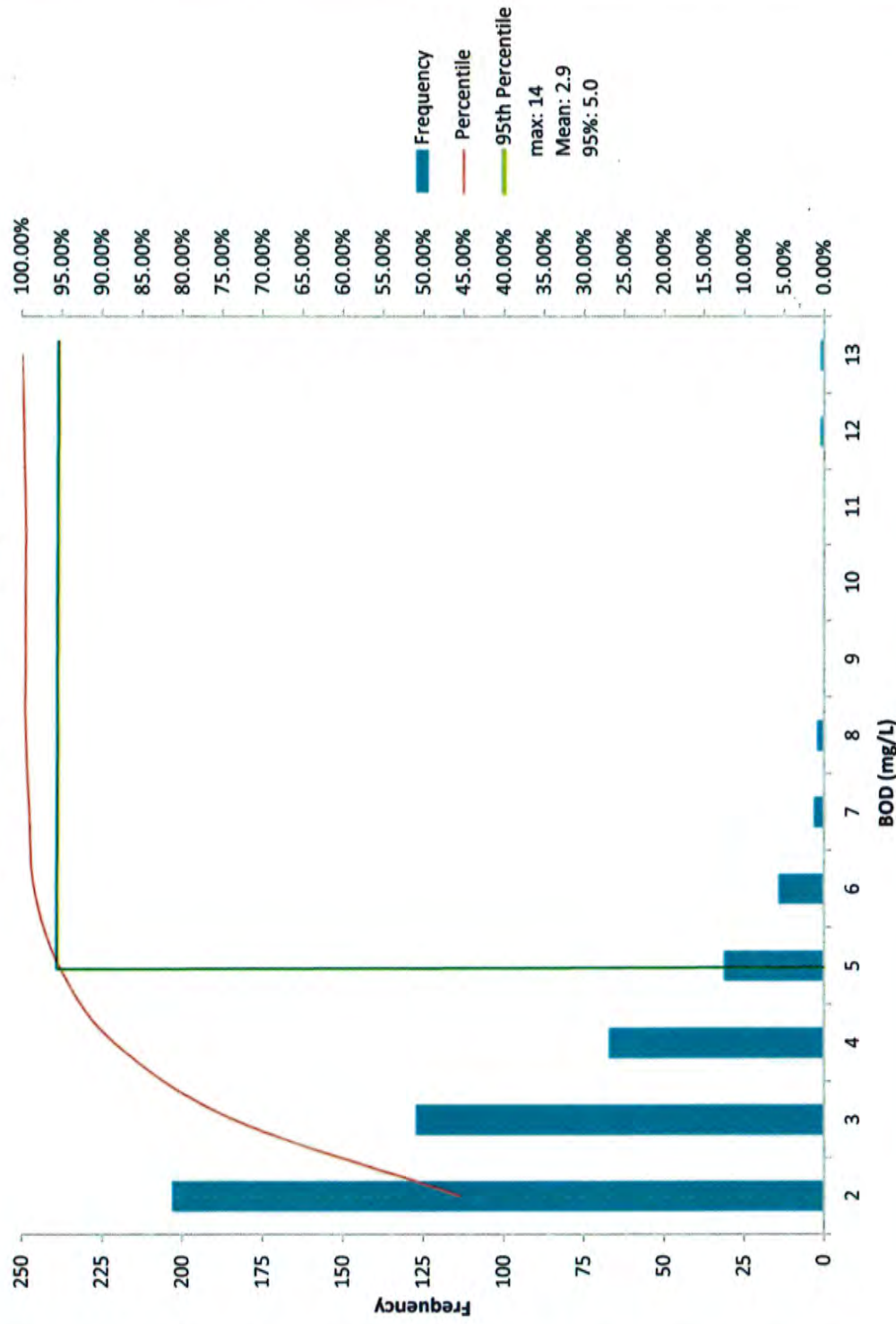
Effluent TSS Histogram



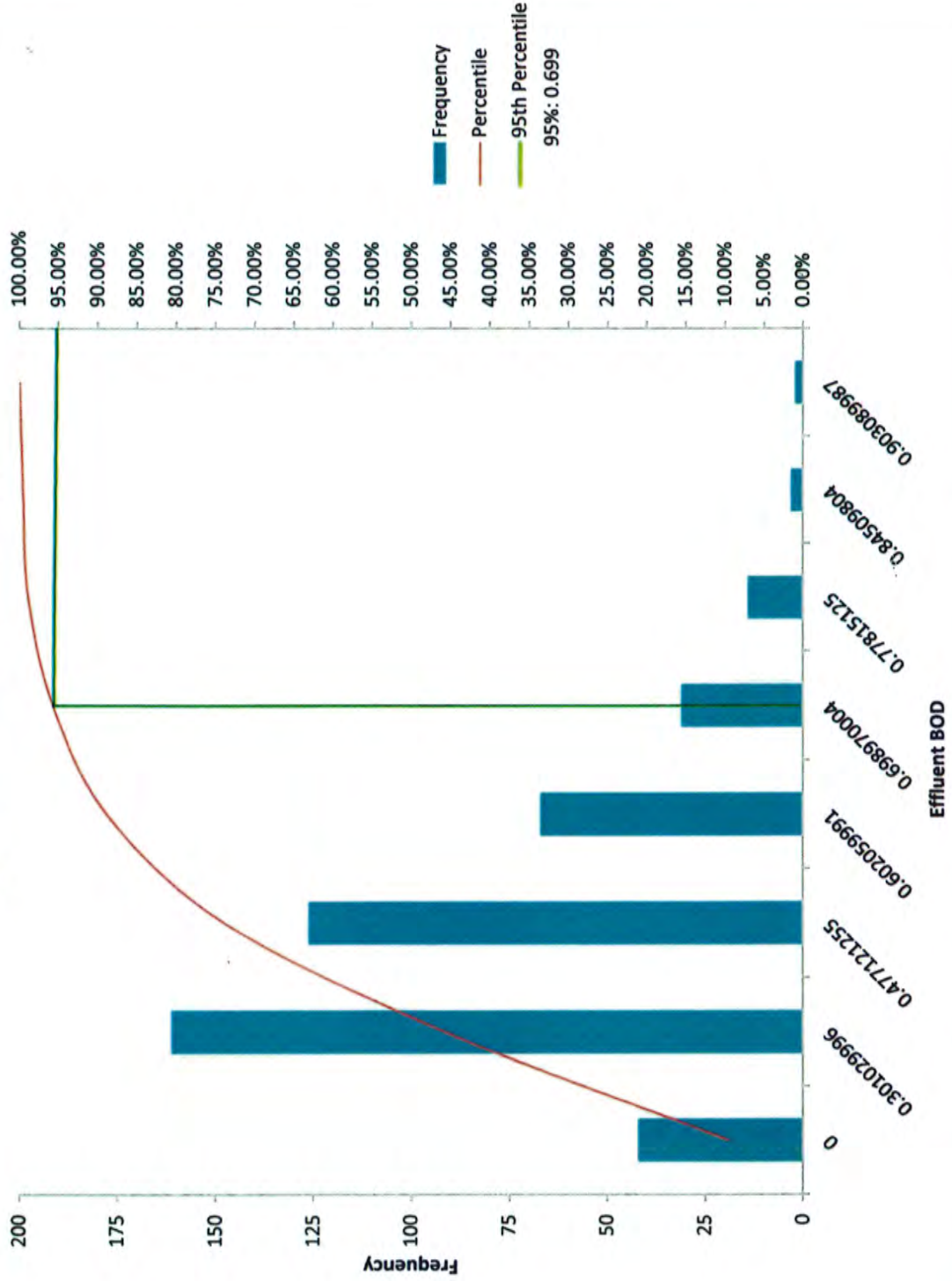
Log Normalized TSS Histogram



Effluent BOD Histogram



Log Normalized BOD Histogram



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CORMIX Model Results 3

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The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

3.49	1.95	0.78	10.0	0.678E+00	0.54	0.134	.37366E+01
7.67	2.39	1.09	16.4	0.414E+00	0.70	0.078	.87385E+01
11.87	2.63	1.41	22.3	0.304E+00	0.82	0.060	.13915E+02
16.07	2.80	1.74	28.4	0.239E+00	0.93	0.049	.19177E+02
20.21	2.92	2.07	34.6	0.196E+00	1.03	0.043	.24427E+02
24.42	3.01	2.40	41.3	0.164E+00	1.13	0.038	.29779E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

28.62	3.09	2.73	48.2	0.141E+00	1.22	0.034	.35161E+02
32.82	3.14	3.04	55.4	0.123E+00	1.31	0.031	.40566E+02
37.02	3.19	3.36	62.8	0.108E+00	1.40	0.028	.45990E+02
41.17	3.23	3.66	70.3	0.965E-01	1.48	0.026	.51353E+02

Cumulative travel time = 51.3532 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
41.17	3.23	3.66	69.4	0.978E-01	1.48	.51353E+02

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
39.69	3.22	5.14	70.4	0.965E-01	0.00	0.00	5.14	5.14	.51353E+02
40.14	3.22	5.14	70.4	0.965E-01	1.84	0.92	5.14	3.30	.51353E+02
40.58	3.23	5.14	70.4	0.965E-01	2.18	1.31	5.14	2.96	.51353E+02
41.03	3.23	5.14	70.3	0.965E-01	2.40	1.60	5.14	2.74	.51353E+02
41.47	3.23	5.14	72.3	0.940E-01	2.56	1.85	5.14	2.58	.51748E+02
41.92	3.23	5.14	81.2	0.836E-01	2.68	2.07	5.14	2.46	.52341E+02
42.36	3.23	5.14	93.6	0.726E-01	2.77	2.26	5.14	2.37	.52933E+02
42.80	3.24	5.14	104.8	0.648E-01	2.84	2.44	5.14	2.30	.53525E+02
43.25	3.24	5.14	112.6	0.603E-01	2.88	2.61	5.14	2.26	.54118E+02
43.69	3.24	5.14	116.8	0.582E-01	2.91	2.77	5.14	2.23	.54710E+02
44.14	3.24	5.14	119.5	0.568E-01	2.92	2.92	5.14	2.22	.55302E+02

Cumulative travel time = 55.3025 sec (0.02 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
44.14	3.24	5.14	119.5	0.568E-01	2.92	2.92	5.14	2.22	.55302E+02
46.78	3.24	5.14	121.4	0.559E-01	2.83	3.07	5.14	2.31	.58831E+02
49.43	3.24	5.14	123.2	0.551E-01	2.75	3.21	5.14	2.39	.62360E+02
52.08	3.24	5.14	125.0	0.543E-01	2.68	3.34	5.14	2.46	.65888E+02
54.72	3.24	5.14	126.9	0.535E-01	2.61	3.48	5.14	2.53	.69417E+02
57.37	3.24	5.14	128.7	0.528E-01	2.55	3.61	5.14	2.59	.72945E+02
60.02	3.24	5.14	130.5	0.520E-01	2.50	3.74	5.14	2.64	.76474E+02
62.66	3.24	5.14	132.4	0.513E-01	2.46	3.87	5.14	2.68	.80003E+02
65.31	3.24	5.14	134.2	0.506E-01	2.41	4.00	5.14	2.73	.83531E+02
67.95	3.24	5.14	136.1	0.499E-01	2.38	4.12	5.14	2.76	.87060E+02
70.60	3.24	5.14	138.0	0.492E-01	2.34	4.24	5.14	2.80	.90588E+02

Cumulative travel time = 90.5885 sec (0.03 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
70.60	-1.00	5.14	138.0	0.492E-01	2.34	8.49	5.14	2.80	.90588E+02
87.88	-1.00	5.14	147.9	0.459E-01	2.31	9.26	5.14	2.83	.11362E+03

CORMIX SESSION REPORT:
 XXX

CORMIX MIXING ZONE EXPERT SYSTEM
 CORMIX Version 11.0GTH
 HYDROL:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
 DESIGN CASE: a. Max Ebb Current, MHHW
 FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\A MHHW max eb current 8 inch port.prd
 Using subsystem CORMIX1: Single Port Discharges
 Start of session: 09/06/2018--16:18:57

 SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section		= unbounded
Average depth	HA	= 4.11 m
Depth at discharge	HD	= 5.14 m
Darcy-Weisbach friction factor	F	= 0.0196
Calculated from Manning's n		= 0.02
Wind velocity	UW	= 5.7 m/s
TIDAL SIMULATION at time	Tsim	= -2.6 hours
Instantaneous ambient velocity	UA	= 0.75 m/s
Maximum tidal velocity	UaMAX	= 1 m/s
Rate of tidal reversal	dUA/dt	= 0.2885 (m/s)/hour
Period of reversal	T	= 12.4 hours
Stratification Type	STRCND	= U
Surface density	RHOAS	= 1012.74 kg/m ³
Bottom density	RHOAB	= 1012.74 kg/m ³

DISCHARGE PARAMETERS:

Single Port Discharge		
Nearest bank		= right
Distance to bank	DISTB	= 1 m
Port diameter	D0	= 0.2032 m
Port cross-sectional area	A0	= 0.0324 m ²
Discharge velocity	U0	= 3.26 m/s
Discharge flowrate	Q0	= 0.10564 m ³ /s
Discharge port height	H0	= 0.31 m
Vertical discharge angle	THETA	= 10 deg
Horizontal discharge angle	SIGMA	= 90 deg
Discharge density	RHO0	= 998 kg/m ³
Density difference	DRHO	= 14.7400 kg/m ³
Buoyant acceleration	GPO	= 0.1427 m/s ²
Discharge concentration	C0	= 6.79 mg/l
Surface heat exchange coeff.	KS	= 0 m/s
Coefficient of decay	KD	= 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m	Lm = 0.78 m	Lb = 0.04 m
LM = 3.66 m	Lm' = 99999 m	Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.1251 hours	Lu = 16.25 m	Lmin= 0.18 m
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NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number	FRO	= 19.13
Velocity ratio	R	= 4.34

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge		= yes
CMC concentration	CMC	= 1.04 mg/l
CCC concentration	CCC	= 0.16 mg/l
Water quality standard specified		= given by CCC value
Regulatory mixing zone		= no
Region of interest		= 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

 | FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
 Applicable layer depth = water depth = 5.14 m

 MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
 1 m from the right bank/shore.
 Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge	c	= 0.0568 mg/l
Dilution at edge of NFR	s	= 119.5
NFR Location:	x	= 44.14 m
(centerline coordinates)	y	= 3.24 m
	z	= 5.14 m

NFR plume dimensions: half-width (bh) = 2.92 m
thickness (bv) = 2.92 m
Cumulative travel time: 55.3024 sec.

Buoyancy assessment:

The effluent density is less than the surrounding ambient water density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:

Plume becomes vertically fully mixed at 419.03 m downstream.

PLUME BANK CONTACT SUMMARY:

Plume in unbounded section contacts nearest bank at 70.60 m downstream.

UNSTEADY TIDAL ASSESSMENT:

Within the region of interest (ROI), the location and trajectory of flow are well represented using steady-state analysis and are not limited by any tidal restrictions.

For this condition BEFORE TIDAL REVERSAL, extensive re-entrainment of previously discharged is unlikely.

To determine the minimum dilution, perform additional simulations after slack tide.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).

Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846

The CMC was encountered at the following plume position:

Plume location: x = 1.71 m
(centerline coordinates) y = 1.61 m
z = 0.64 m

Plume dimension: half-width (bh) = 0.06 m
thickness (bv) = 0.06 m

Computed distance from port opening to CMC location = 2.37 m.

CRITERION 1: This location is within 50 times the discharge length scale of
Lq = 0.18 m.

+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 2.35 m.

CRITERION 2: This location is within 5 times the ambient water depth of
HD = 5.14 m.

+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***

***** REGULATORY MIXING ZONE SUMMARY *****

No RMZ has been specified.

However:

The CCC was encountered at the following plume position:

The CCC for the toxic pollutant was encountered at the following plume position:

CCC = 0.16 mg/l
Corresponding dilution = 42.4
Plume location: x = 25.13 m
(centerline coordinates) y = 3.03 m
z = 2.46 m

Computed horizontal distance from port opening to CCC location = 25.40

Plume dimension: half-width (bh) = 1.14 m

***** FINAL DESIGN ADVICE AND COMMENTS *****

REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.

Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +/-50% (standard deviation).

As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

0.02 0.51 0.39 1.0 0.679E+01 0.11 3.257 .94232E-02
0.86 2.00 0.71 4.4 0.154E+01 0.40 0.599 .10613E+01
**** CMC HAS BEEN FOUND ****
The pollutant concentration in the plume falls below CMC value of 0.104E+01
in the current prediction interval.
This is the extent of the TOXIC DILUTION ZONE.

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
2.50	2.70	0.95	9.2	0.739E+00	0.62	0.217	.33516E+01		
4.29	3.09	1.17	13.3	0.512E+00	0.75	0.140	.61743E+01		
6.07	3.35	1.39	16.9	0.402E+00	0.86	0.110	.91208E+01		
7.89	3.55	1.62	20.5	0.331E+00	0.95	0.092	.12239E+02		
9.68	3.71	1.85	24.0	0.282E+00	1.03	0.081	.15355E+02		
11.51	3.84	2.08	27.7	0.245E+00	1.11	0.073	.18591E+02		
13.30	3.94	2.31	31.4	0.216E+00	1.19	0.067	.21794E+02		
15.13	4.03	2.55	35.3	0.193E+00	1.26	0.062	.25100E+02		
16.93	4.11	2.78	39.1	0.174E+00	1.33	0.057	.28346E+02		

Cumulative travel time = 28.3460 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
16.93	4.11	2.78	39.0	0.174E+00	1.33	.28346E+02

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
15.60	4.08	4.11	39.1	0.174E+00	0.00	0.00	4.11	4.11	.28346E+02
16.00	4.09	4.11	39.1	0.174E+00	1.67	0.84	4.11	2.44	.28346E+02
16.39	4.10	4.11	39.1	0.174E+00	1.98	1.19	4.11	2.13	.28346E+02
16.79	4.10	4.11	39.1	0.174E+00	2.18	1.45	4.11	1.93	.28346E+02
17.19	4.11	4.11	40.2	0.169E+00	2.32	1.68	4.11	1.79	.28879E+02

****WATER QUALITY STANDARD OR CCC HAS BEEN FOUND****

The pollutant concentration in the plume falls below water quality standard
or CCC value of 0.160E+00 in the current prediction interval.
This is the spatial extent of concentrations exceeding the water quality
standard or CCC value.

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
17.59	4.12	4.11	45.2	0.150E+00	2.43	1.88	4.11	1.68	.29676E+02
17.99	4.13	4.11	52.1	0.130E+00	2.52	2.06	4.11	1.59	.30473E+02
18.39	4.13	4.11	58.3	0.116E+00	2.58	2.22	4.11	1.53	.31269E+02
18.79	4.14	4.11	62.6	0.108E+00	2.62	2.37	4.11	1.49	.32066E+02
19.18	4.15	4.11	65.0	0.105E+00	2.65	2.52	4.11	1.46	.32863E+02
19.58	4.16	4.11	66.5	0.102E+00	2.65	2.65	4.11	1.46	.33660E+02

Cumulative travel time = 33.6603 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

**** End of NEAR-FIELD REGION (NFR) ****

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
19.58	4.16	4.11	66.5	0.102E+00	2.65	2.65	4.11	1.46	.33660E+02
22.43	4.16	4.11	68.3	0.994E-01	2.46	2.95	4.11	1.65	.39344E+02
25.27	4.16	4.11	70.0	0.970E-01	2.30	3.23	4.11	1.81	.45028E+02
28.11	4.16	4.11	71.5	0.950E-01	2.17	3.50	4.11	1.94	.50712E+02
30.96	4.16	4.11	72.9	0.931E-01	2.06	3.76	4.11	2.05	.56396E+02
33.80	4.16	4.11	74.3	0.914E-01	1.97	4.01	4.11	2.14	.62079E+02
36.64	4.16	4.11	75.5	0.899E-01	1.89	4.25	4.11	2.22	.67763E+02
39.48	4.16	4.11	76.8	0.884E-01	1.82	4.49	4.11	2.29	.73447E+02
42.33	4.16	4.11	78.0	0.871E-01	1.76	4.72	4.11	2.35	.79131E+02
45.17	4.16	4.11	79.1	0.858E-01	1.71	4.94	4.11	2.40	.84815E+02
48.01	4.16	4.11	80.3	0.846E-01	1.66	5.16	4.11	2.45	.90498E+02

Cumulative travel time = 90.4984 sec (0.03 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
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CORMIX SESSION REPORT:
XX

CORMIX MIXING ZONE EXPERT SYSTEM
CORMIX Version 11.0GTH
HYDRO1:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: b. mid High and Low Slack
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\b mid High and Low Slack single diff 8inch port.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:21:12

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = bounded
Width BS = 116 m
Channel regularity ICHREG = 1
Ambient flowrate QA = 238.38 m³/s
Average depth HA = 4.11 m
Depth at discharge HD = 4.11 m
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = -1.1 hours
Instantaneous ambient velocity UA = 0.5 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.4545 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m³
Bottom density RHOAB = 1012.74 kg/m³

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter D0 = 0.2032 m
Port cross-sectional area A0 = 0.0324 m²
Discharge velocity U0 = 3.26 m/s
Discharge flowrate Q0 = 0.105613 m³/s
Discharge port height H0 = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHOO = 998 kg/m³
Density difference DRHO = 14.7400 kg/m³
Buoyant acceleration GP0 = 0.1427 m/s²
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 1.17 m Lb = 0.12 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.0924 hours Lu = 13.97 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 6.51

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 4.11 m

Limiting Dilution S = (QA/Q0)+ 1.0 = 2258.1

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the

discharge design conditions.

Pollutant concentration at NFR edge c = 0.1021 mg/l
Dilution at edge of NFR s = 66.5
NFR Location: x = 19.58 m
(centerline coordinates) y = 4.16 m
z = 4.11 m
NFR plume dimensions: half-width (bh) = 2.65 m
thickness (bv) = 2.65 m
Cumulative travel time: 33.6603 sec.

Buoyancy assessment:

The effluent density is less than the surrounding ambient water density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:

Plume becomes vertically fully mixed at 850.15 m downstream.

PLUME BANK CONTACT SUMMARY:

Plume in bounded section contacts one bank only at 48.01 m downstream.

UNSTEADY TIDAL ASSESSMENT:

Because of the unsteadiness of the ambient current during the tidal reversal, CORMIX predictions have been TERMINATED at:
x = 869.79 m
y = -1 m
z = 4.11 m.

For this condition BEFORE TIDAL REVERSAL, extensive re-entrainment of previously discharged is unlikely.
To determine the minimum dilution, perform additional simulations after slack tide.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).

Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846
The CMC was encountered at the following plume position:
Plume location: x = 1.51 m
(centerline coordinates) y = 2.36 m
z = 0.82 m
Plume dimension: half-width (bh) = 0.04 m
thickness (bv) = 0.04 m

Computed distance from port opening to CMC location = 2.85 m.
CRITERION 1: This location is within 50 times the discharge length scale of Lq = 0.18 m.
+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 2.80 m.
CRITERION 2: This location is within 5 times the ambient water depth of HD = 4.11 m.
+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***
***** REGULATORY MIXING ZONE SUMMARY *****
No RMZ has been specified.

However:

The CCC was encountered at the following plume position:
The CCC for the toxic pollutant was encountered at the following plume position:
CCC = 0.16 mg/l
Corresponding dilution = 42.6
Plume location: x = 17.38 m
(centerline coordinates) y = 4.12 m
z = 4.11 m

Computed horizontal distance from port opening to CCC location = 18.26
Plume dimensions: half-width (bh) = 1.77 m
thickness (bv) = 2.38 m

***** FINAL DESIGN ADVICE AND COMMENTS *****

REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.
Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +-50% (standard deviation).
As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

CORMIX SESSION REPORT:
XX

CORMIX MIXING ZONE EXPERT SYSTEM
CORMIX Version 11.0GTH
HYDRO1:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: c. Low Slack
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\c Low Slack single diff 8 inch port.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:25:41

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 3.14 m
Ambient velocity UA = 0 m/s
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m³
Bottom density RHOAB = 1012.74 kg/m³

DISCHARGE PARAMETERS: Single Port Discharge

Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter D0 = 0.2032 m
Port cross-sectional area A0 = 0.0324 m²
Discharge velocity U0 = 3.26 m/s
Discharge flowrate Q0 = 0.105613 m³/s
Discharge port height H0 = 0.6 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m³
Density difference DRHO = 14.7400 kg/m³
Buoyant acceleration GP0 = 0.1427 m/s²
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 99999 m Lb = 99999 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 99999

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H4-90 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.

Applicable layer depth = water depth = 3.14 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge c = 0.9543 mg/l
Dilution at edge of NFR s = 7.1
NFR Location: x = 0 m
(centerline coordinates) y = 7.23 m
z = 3.14 m

NFR plume dimensions: half-width (bh) = 0.95 m
thickness (bv) = 0.95 m

Cumulative travel time: 7.2217 sec.

Buoyancy assessment:

The effluent density is less than the surrounding ambient water

density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:

Because of the specified STAGNANT ambient conditions, there exists no steady-state far-field for this discharge.
Unsteady circulations and pollutant build-up may result in the far-field.

PLUME BANK CONTACT SUMMARY:

Plume in unbounded section does not contact bank in this simulation.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).

Criterion maximum concentration (CMC) = 1.04 mg/l

Corresponding dilution = 6.528846

The CMC was encountered at the following plume position:

Plume location: x = -0.01 m

(centerline coordinates) y = 6.45 m

z = 3.14 m

Plume dimension: half-width (bh) = 0.61 m

thickness (bv) = 0.84 m

Computed distance from port opening to CMC location = 6.93 m.

CRITERION 1: This location is within 50 times the discharge length scale of

Lq = 0.18 m.

+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 6.45 m.

CRITERION 2: This location is within 5 times the ambient water depth of

HD = 3.14 m.

+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.

This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***

***** REGULATORY MIXING ZONE SUMMARY *****

No RMZ has been specified.

The CCC for the toxic pollutant was not encountered within the predicted plume region.

***** FINAL DESIGN ADVICE AND COMMENTS *****

REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.

Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +/-50% (standard deviation).

As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

** CMC HAS BEEN FOUND **

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

3.20	1.76	0.72	9.2	0.737E+00	0.51	0.137	.32194E+01
5.07	2.00	0.85	12.1	0.559E+00	0.58	0.098	.53055E+01
6.98	2.17	0.98	14.8	0.459E+00	0.65	0.080	.74779E+01
8.89	2.30	1.11	17.3	0.392E+00	0.70	0.068	.96832E+01
10.77	2.40	1.25	19.8	0.343E+00	0.75	0.061	.11879E+02
12.68	2.49	1.38	22.3	0.304E+00	0.80	0.055	.14123E+02
14.59	2.57	1.52	24.9	0.273E+00	0.85	0.050	.16380E+02
16.47	2.63	1.66	27.4	0.248E+00	0.89	0.047	.18615E+02
18.39	2.69	1.80	30.0	0.226E+00	0.93	0.043	.20869E+02

Cumulative travel time = 20.8685 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
18.39	2.69	1.80	29.5	0.230E+00	0.93	.20869E+02

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
17.45	2.67	2.74	30.0	0.226E+00	0.00	0.00	2.74	2.74	.20869E+02
17.73	2.68	2.74	30.0	0.226E+00	1.16	0.59	2.74	1.58	.20869E+02
18.01	2.68	2.74	30.0	0.226E+00	1.38	0.83	2.74	1.36	.20869E+02
18.29	2.68	2.74	30.0	0.226E+00	1.52	1.01	2.74	1.22	.20869E+02
18.57	2.69	2.74	30.8	0.220E+00	1.62	1.17	2.74	1.12	.21103E+02
18.85	2.69	2.74	34.6	0.196E+00	1.70	1.31	2.74	1.04	.21453E+02
19.13	2.70	2.74	39.9	0.170E+00	1.76	1.43	2.74	0.98	.21804E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

19.42	2.70	2.74	44.7	0.152E+00	1.80	1.55	2.74	0.94	.22154E+02
19.70	2.70	2.74	48.0	0.142E+00	1.83	1.66	2.74	0.91	.22505E+02
19.98	2.71	2.74	49.8	0.136E+00	1.85	1.76	2.74	0.89	.22855E+02
20.26	2.71	2.74	50.9	0.133E+00	1.85	1.85	2.74	0.89	.23206E+02

Cumulative travel time = 23.2059 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
20.26	2.71	2.74	50.9	0.133E+00	1.85	1.85	2.74	0.89	.23206E+02
23.86	2.71	2.74	52.6	0.129E+00	1.71	2.07	2.74	1.03	.27709E+02
27.46	2.71	2.74	54.2	0.125E+00	1.61	2.28	2.74	1.13	.32212E+02
31.06	2.71	2.74	55.8	0.122E+00	1.53	2.48	2.74	1.21	.36716E+02
34.67	2.71	2.74	57.4	0.118E+00	1.46	2.67	2.74	1.28	.41219E+02
38.27	2.71	2.74	59.0	0.115E+00	1.41	2.86	2.74	1.33	.45722E+02
41.87	2.71	2.74	60.7	0.112E+00	1.37	3.04	2.74	1.37	.50225E+02
45.47	2.71	2.74	62.4	0.109E+00	1.34	3.21	2.74	1.40	.54729E+02
49.08	2.71	2.74	64.2	0.106E+00	1.31	3.38	2.74	1.43	.59232E+02
52.68	2.71	2.74	66.1	0.103E+00	1.29	3.55	2.74	1.45	.63735E+02
56.28	2.71	2.74	68.0	0.999E-01	1.27	3.71	2.74	1.47	.68238E+02

Cumulative travel time = 68.2382 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
56.28	-1.00	2.74	68.0	0.999E-01	1.27	7.42	2.74	1.47	.68238E+02
67.12	-1.00	2.74	73.1	0.929E-01	1.30	7.90	2.74	1.44	.81789E+02

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

5.91	1.54	0.74	11.9	0.568E+00	0.52	0.069	.53491E+01
12.27	1.88	1.09	18.9	0.359E+00	0.66	0.045	.11414E+02
18.63	2.07	1.45	25.8	0.263E+00	0.78	0.035	.17563E+02
25.05	2.19	1.82	33.2	0.204E+00	0.89	0.029	.23807E+02
31.42	2.28	2.18	40.9	0.166E+00	0.99	0.025	.30029E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

37.78	2.35	2.53	49.0	0.138E+00	1.08	0.022	.36271E+02
44.20	2.40	2.87	57.5	0.118E+00	1.18	0.019	.42580E+02
50.57	2.43	3.20	66.2	0.103E+00	1.26	0.018	.48850E+02
56.94	2.47	3.52	75.1	0.904E-01	1.35	0.016	.55129E+02
63.36	2.49	3.83	84.2	0.806E-01	1.43	0.015	.61431E+02

Cumulative travel time = 61.4312 sec (0.02 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
63.36	2.49	3.83	80.1	0.848E-01	1.43	.61431E+02

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
61.93	2.49	5.27	84.3	0.805E-01	0.00	0.00	5.27	5.27	.61431E+02
62.36	2.49	5.27	84.3	0.805E-01	1.77	0.89	5.27	3.50	.61431E+02
62.79	2.49	5.27	84.3	0.806E-01	2.10	1.26	5.27	3.17	.61431E+02
63.22	2.49	5.27	84.2	0.806E-01	2.31	1.54	5.27	2.96	.61431E+02
63.65	2.49	5.27	86.5	0.785E-01	2.47	1.78	5.27	2.80	.61718E+02
64.08	2.49	5.27	97.2	0.698E-01	2.59	1.99	5.27	2.68	.62148E+02
64.51	2.49	5.27	112.0	0.606E-01	2.67	2.18	5.27	2.60	.62579E+02
64.94	2.49	5.27	125.5	0.541E-01	2.74	2.36	5.27	2.53	.63009E+02
65.37	2.50	5.27	134.7	0.504E-01	2.78	2.52	5.27	2.49	.63440E+02
65.80	2.50	5.27	139.7	0.486E-01	2.81	2.68	5.27	2.46	.63870E+02
66.23	2.50	5.27	142.9	0.475E-01	2.82	2.82	5.27	2.45	.64300E+02

Cumulative travel time = 64.3004 sec (0.02 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
BH = top-hat half-width, measured horizontally in Y-direction
ZU = upper plume boundary (Z-coordinate)
ZL = lower plume boundary (Z-coordinate)
S = hydrodynamic average (bulk) dilution
C = average (bulk) concentration (includes reaction effects, if any)
TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
66.23	2.50	5.27	142.9	0.475E-01	2.82	2.82	5.27	2.45	.64300E+02
68.19	2.50	5.27	144.7	0.469E-01	2.79	2.89	5.27	2.48	.66261E+02
70.15	2.50	5.27	146.4	0.464E-01	2.76	2.96	5.27	2.51	.68222E+02
72.11	2.50	5.27	148.3	0.458E-01	2.73	3.03	5.27	2.54	.70182E+02
74.07	2.50	5.27	150.1	0.452E-01	2.71	3.10	5.27	2.56	.72143E+02
76.03	2.50	5.27	152.0	0.447E-01	2.69	3.17	5.27	2.58	.74103E+02
77.99	2.50	5.27	153.9	0.441E-01	2.67	3.24	5.27	2.60	.76064E+02
79.95	2.50	5.27	155.9	0.436E-01	2.65	3.30	5.27	2.62	.78025E+02
81.91	2.50	5.27	157.8	0.430E-01	2.64	3.37	5.27	2.63	.79985E+02
83.88	2.50	5.27	159.9	0.425E-01	2.63	3.43	5.27	2.64	.81946E+02
85.84	2.50	5.27	162.0	0.419E-01	2.61	3.50	5.27	2.66	.83907E+02

Cumulative travel time = 83.9066 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Discharge is non-buoyant or weakly buoyant.
Therefore BUOYANT SPREADING REGIME is ABSENT.

END OF MOD141: BUOYANT AMBIENT SPREADING

NFR plume dimensions: half-width (bh) = 2.82 m
thickness (bv) = 2.82 m
Cumulative travel time: 64.3004 sec.

Buoyancy assessment:
The effluent density is less than the surrounding ambient water density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:
Plume becomes vertically fully mixed at 277.25 m downstream.

PLUME BANK CONTACT SUMMARY:
Plume in unbounded section contacts nearest bank at 85.84 m downstream.

UNSTEADY TIDAL ASSESSMENT:
Because of the unsteadiness of the ambient current during the tidal reversal, CORMIX predictions have been TERMINATED at:

x = 1718.87 m
y = -1 m
z = 5.27 m.

For this condition AFTER TIDAL REVERSAL, mixed water from the previous half-cycle becomes re-entrained into the near field of the discharge, increasing pollutant concentrations compared to steady-state predictions. A pool of mixed water formed at slack tide will be advected downstream in this phase.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).

Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846
The CMC was encountered at the following plume position:
Plume location: x = 1.98 m
(centerline coordinates) y = 1.09 m
z = 0.54 m
Plume dimension: half-width (bh) = 0.09 m
thickness (bv) = 0.09 m

Computed distance from port opening to CMC location = 2.28 m.
CRITERION 1: This location is within 50 times the discharge length scale of $L_q = 0.18$ m.
++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 2.27 m.
CRITERION 2: This location is within 5 times the ambient water depth of $HD = 5.27$ m.
+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***
***** REGULATORY MIXING ZONE SUMMARY *****
No RMZ has been specified.

However:
The CCC was encountered at the following plume position:
The CCC for the toxic pollutant was encountered at the following plume position:
CCC = 0.16 mg/l
Corresponding dilution = 42.4
Plume location: x = 32.62 m
(centerline coordinates) y = 2.29 m
z = 2.24 m

Computed horizontal distance from port opening to CCC location = 32.75
Plume dimension: half-width (bh) = 1.00 m

***** FINAL DESIGN ADVICE AND COMMENTS *****

REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.

Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about $\pm 50\%$ (standard deviation).

As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

2.88	1.86	0.73	8.9	0.762E+00	0.51	0.155	.30218E+01
6.38	2.28	0.99	14.5	0.467E+00	0.66	0.088	.71645E+01
9.95	2.53	1.26	19.6	0.346E+00	0.77	0.067	.11529E+02
13.48	2.71	1.54	24.6	0.276E+00	0.87	0.055	.15917E+02
17.01	2.83	1.82	29.8	0.228E+00	0.95	0.048	.20353E+02
20.58	2.93	2.10	35.2	0.193E+00	1.04	0.042	.24881E+02
24.11	3.01	2.38	40.8	0.166E+00	1.12	0.038	.29379E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

27.64	3.07	2.65	46.7	0.146E+00	1.20	0.034	.33898E+02
31.22	3.12	2.92	52.7	0.129E+00	1.28	0.032	.38492E+02
34.75	3.17	3.19	58.9	0.115E+00	1.35	0.029	.43019E+02

Cumulative travel time = 43.0189 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
34.75	3.17	3.19	58.4	0.116E+00	1.35	.43019E+02

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
33.40	3.16	4.54	58.9	0.115E+00	0.00	0.00	4.54	4.54	.43019E+02
33.80	3.16	4.54	58.9	0.115E+00	1.68	0.84	4.54	2.86	.43019E+02
34.21	3.16	4.54	58.9	0.115E+00	1.99	1.19	4.54	2.55	.43019E+02
34.61	3.16	4.54	58.9	0.115E+00	2.19	1.46	4.54	2.35	.43019E+02
35.02	3.17	4.54	60.5	0.112E+00	2.33	1.69	4.54	2.21	.43379E+02
35.42	3.17	4.54	68.0	0.999E-01	2.44	1.88	4.54	2.10	.43919E+02
35.83	3.17	4.54	78.3	0.867E-01	2.53	2.06	4.54	2.01	.44459E+02
36.23	3.17	4.54	87.7	0.774E-01	2.59	2.23	4.54	1.95	.45000E+02
36.64	3.18	4.54	94.2	0.721E-01	2.63	2.38	4.54	1.91	.45540E+02
37.04	3.18	4.54	97.7	0.695E-01	2.66	2.53	4.54	1.88	.46080E+02
37.45	3.18	4.54	100.0	0.679E-01	2.67	2.67	4.54	1.87	.46620E+02

Cumulative travel time = 46.6203 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
37.45	3.18	4.54	100.0	0.679E-01	2.67	2.67	4.54	1.87	.46620E+02
40.41	3.18	4.54	102.0	0.666E-01	2.56	2.83	4.54	1.98	.50574E+02
43.38	3.18	4.54	103.9	0.653E-01	2.47	3.00	4.54	2.07	.54527E+02
46.34	3.18	4.54	105.8	0.642E-01	2.39	3.16	4.54	2.15	.58480E+02
49.31	3.18	4.54	107.7	0.630E-01	2.32	3.31	4.54	2.22	.62434E+02
52.27	3.18	4.54	109.6	0.619E-01	2.26	3.47	4.54	2.28	.66387E+02
55.24	3.18	4.54	111.5	0.609E-01	2.20	3.62	4.54	2.34	.70340E+02
58.20	3.18	4.54	113.5	0.598E-01	2.15	3.76	4.54	2.39	.74294E+02
61.17	3.18	4.54	115.4	0.588E-01	2.11	3.90	4.54	2.43	.78247E+02
64.13	3.18	4.54	117.4	0.578E-01	2.08	4.04	4.54	2.46	.82200E+02
67.10	3.18	4.54	119.4	0.569E-01	2.04	4.18	4.54	2.50	.86154E+02

Cumulative travel time = 86.1535 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
67.10	-1.00	4.54	119.4	0.569E-01	2.04	8.37	4.54	2.50	.86154E+02
83.19	-1.00	4.54	128.3	0.529E-01	2.03	9.09	4.54	2.51	.10761E+03

CORMIX SESSION REPORT:

XX

CORMIX MIXING ZONE EXPERT SYSTEM

CORMIX Version 11.0GTH

HYDROL:Version-11.0.0.0 April, 2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: f. Mid MLLW flood to MHHW
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\f midMLLWfloodtoMHHW8inch port.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:34:06

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 4.54 m
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = -1.2 hours
Instantaneous ambient velocity UA = 0.75 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.625 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter DO = 0.2032 m
Port cross-sectional area AO = 0.0324 m^2
Discharge velocity UO = 3.26 m/s
Discharge flowrate QO = 0.105613 m^3/s
Discharge port height HO = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 0.78 m Lb = 0.04 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.0747 hours Lu = 12.56 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 4.34

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 4.54 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.
Pollutant concentration at NFR edge c = 0.0679 mg/l
Dilution at edge of NFR s = 100.0
NFR Location: x = 37.45 m
(centerline coordinates) y = 3.18 m
z = 4.54 m

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

2.88	1.86	0.73	8.9	0.762E+00	0.51	0.155	.30218E+01
6.38	2.28	0.99	14.5	0.467E+00	0.66	0.088	.71645E+01
9.95	2.53	1.26	19.6	0.346E+00	0.77	0.067	.11529E+02
13.48	2.71	1.54	24.6	0.276E+00	0.87	0.055	.15917E+02
17.01	2.83	1.82	29.8	0.228E+00	0.95	0.048	.20353E+02
20.58	2.93	2.10	35.2	0.193E+00	1.04	0.042	.24881E+02
24.11	3.01	2.38	40.8	0.166E+00	1.12	0.038	.29379E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

27.64	3.07	2.65	46.7	0.146E+00	1.20	0.034	.33898E+02
31.22	3.12	2.92	52.7	0.129E+00	1.28	0.032	.38492E+02
34.75	3.17	3.19	58.9	0.115E+00	1.35	0.029	.43019E+02

Cumulative travel time = 43.0189 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
34.75	3.17	3.19	58.4	0.116E+00	1.35	.43019E+02

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
33.40	3.16	4.54	58.9	0.115E+00	0.00	0.00	4.54	4.54	.43019E+02
33.80	3.16	4.54	58.9	0.115E+00	1.68	0.84	4.54	2.86	.43019E+02
34.21	3.16	4.54	58.9	0.115E+00	1.99	1.19	4.54	2.55	.43019E+02
34.61	3.16	4.54	58.9	0.115E+00	2.19	1.46	4.54	2.35	.43019E+02
35.02	3.17	4.54	60.5	0.112E+00	2.33	1.69	4.54	2.21	.43379E+02
35.42	3.17	4.54	68.0	0.999E-01	2.44	1.88	4.54	2.10	.43919E+02
35.83	3.17	4.54	78.3	0.867E-01	2.53	2.06	4.54	2.01	.44459E+02
36.23	3.17	4.54	87.7	0.774E-01	2.59	2.23	4.54	1.95	.45000E+02
36.64	3.18	4.54	94.2	0.721E-01	2.63	2.38	4.54	1.91	.45540E+02
37.04	3.18	4.54	97.7	0.695E-01	2.66	2.53	4.54	1.88	.46080E+02
37.45	3.18	4.54	100.0	0.679E-01	2.67	2.67	4.54	1.87	.46620E+02

Cumulative travel time = 46.6203 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
37.45	3.18	4.54	100.0	0.679E-01	2.67	2.67	4.54	1.87	.46620E+02
40.41	3.18	4.54	102.0	0.666E-01	2.56	2.83	4.54	1.98	.50574E+02
43.38	3.18	4.54	103.9	0.653E-01	2.47	3.00	4.54	2.07	.54527E+02
46.34	3.18	4.54	105.8	0.642E-01	2.39	3.16	4.54	2.15	.58480E+02
49.31	3.18	4.54	107.7	0.630E-01	2.32	3.31	4.54	2.22	.62434E+02
52.27	3.18	4.54	109.6	0.619E-01	2.26	3.47	4.54	2.28	.66387E+02
55.24	3.18	4.54	111.5	0.609E-01	2.20	3.62	4.54	2.34	.70340E+02
58.20	3.18	4.54	113.5	0.598E-01	2.15	3.76	4.54	2.39	.74294E+02
61.17	3.18	4.54	115.4	0.588E-01	2.11	3.90	4.54	2.43	.78247E+02
64.13	3.18	4.54	117.4	0.578E-01	2.08	4.04	4.54	2.46	.82200E+02
67.10	3.18	4.54	119.4	0.569E-01	2.04	4.18	4.54	2.50	.86154E+02

Cumulative travel time = 86.1535 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
67.10	-1.00	4.54	119.4	0.569E-01	2.04	8.37	4.54	2.50	.86154E+02
83.19	-1.00	4.54	128.3	0.529E-01	2.03	9.09	4.54	2.51	.10761E+03

CORMIX SESSION REPORT:

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CORMIX MIXING ZONE EXPERT SYSTEM

CORMIX Version 11.0GTH

HYDRO1:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: f. Mid MLLW flood to MHHW
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\f midMLLWfloodtoMHHW8inch port.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:35:23

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 4.54 m
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = -1.2 hours
Instantaneous ambient velocity UA = 0.75 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.625 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter DO = 0.2032 m
Port cross-sectional area AO = 0.0324 m^2
Discharge velocity UO = 3.26 m/s
Discharge flowrate QO = 0.105613 m^3/s
Discharge port height HO = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHOO = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 0.78 m Lb = 0.04 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.0747 hours Lu = 12.56 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 4.34

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 4.54 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge c = 0.0679 mg/l
Dilution at edge of NFR s = 100.0
NFR Location: x = 37.45 m
(centerline coordinates) y = 3.18 m
z = 4.54 m

NFR plume dimensions: half-width (bh) = 2.67 m
thickness (bv) = 2.67 m
Cumulative travel time: 46.6202 sec.

Buoyancy assessment:
The effluent density is less than the surrounding ambient water density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:
Plume becomes vertically fully mixed at 405.24 m downstream.

PLUME BANK CONTACT SUMMARY:
Plume in unbounded section contacts nearest bank at 67.10 m downstream.

UNSTEADY TIDAL ASSESSMENT:
Because of the unsteadiness of the ambient current during the tidal reversal, CORMIX predictions have been TERMINATED at:
x = 1381.60 m
y = -1 m
z = 4.54 m.

For this condition BEFORE TIDAL REVERSAL, extensive re-entrainment of previously discharged is unlikely.
To determine the minimum dilution, perform additional simulations after slack tide.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).
Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846
The CMC was encountered at the following plume position:
Plume location: x = 1.72 m
(centerline coordinates) y = 1.61 m
z = 0.64 m
Plume dimension: half-width (bh) = 0.08 m
thickness (bv) = 0.08 m

Computed distance from port opening to CMC location = 2.38 m.
CRITERION 1: This location is within 50 times the discharge length scale of Lq = 0.18 m.
+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 2.35 m.
CRITERION 2: This location is within 5 times the ambient water depth of HD = 4.54 m.
+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***
***** REGULATORY MIXING ZONE SUMMARY *****
No RMZ has been specified.

However:
The CCC was encountered at the following plume position:
The CCC for the toxic pollutant was encountered at the following plume position:
plume position:
CCC = 0.16 mg/l
Corresponding dilution = 42.4
Plume location: x = 25.09 m
(centerline coordinates) y = 3.03 m
z = 2.45 m

Computed horizontal distance from port opening to CCC location = 25.36
Plume dimension: half-width (bh) = 1.14 m

***** FINAL DESIGN ADVICE AND COMMENTS *****

REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.
Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +/-50% (standard deviation).
As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

CORMIX SESSION REPORT:

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CORMIX MIXING ZONE EXPERT SYSTEM

CORMIX Version 11.0GTH

HYDR01:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: g. High Water Slack
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\g HighWaterSlack8inchport.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:37:07

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 4.99 m
Ambient velocity UA = 0 m/s
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter D0 = 0.2032 m
Port cross-sectional area A0 = 0.0324 m^2
Discharge velocity U0 = 3.26 m/s
Discharge flowrate Q0 = 0.105613 m^3/s
Discharge port height H0 = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration C0 = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 99999 m Lb = 99999 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 99999

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 2000 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H4-90 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 4.99 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge c = 0.5091 mg/l
Dilution at edge of NFR s = 13.3
NFR Location: x = 0 m
(centerline coordinates) y = 9.99 m
z = 4.99 m
NFR plume dimensions: half-width (bh) = 1.37 m
thickness (bv) = 1.37 m

Cumulative travel time: 14.2160 sec.

Buoyancy assessment:

The effluent density is less than the surrounding ambient water

density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:

Because of the specified STAGNANT ambient conditions, there exists no steady-state far-field for this discharge.
Unsteady circulations and pollutant build-up may result in the far-field.

PLUME BANK CONTACT SUMMARY:

Plume in unbounded section does not contact bank in this simulation.
***** TOXIC DILUTION ZONE SUMMARY *****
Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).
Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846
The CMC was encountered at the following plume position:
Plume location: x = 0 m
(centerline coordinates) y = 6.29 m
z = 2.43 m
Plume dimension: half-width (bh) = 0.07 m
thickness (bv) = 0.07 m

Computed distance from port opening to CMC location = 6.64 m.
CRITERION 1: This location is within 50 times the discharge length scale of
Lq = 0.18 m.
+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 6.29 m.
CRITERION 2: This location is within 5 times the ambient water depth of
HD = 4.99 m.
+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***
***** REGULATORY MIXING ZONE SUMMARY *****
No RMZ has been specified.

The CCC for the toxic pollutant was not encountered within the predicted plume region.
***** FINAL DESIGN ADVICE AND COMMENTS *****
REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.
Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +/-50% (standard deviation).
As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

0.05	1.92	0.66	1.8	0.373E+01	0.22	2.118	.37085E+00
0.10	2.44	0.78	2.3	0.291E+01	0.28	1.659	.65847E+00
0.16	2.96	0.92	2.9	0.236E+01	0.34	1.358	.10152E+01
0.24	3.46	1.07	3.4	0.198E+01	0.41	1.145	.14419E+01
0.34	3.96	1.24	4.0	0.168E+01	0.47	0.986	.19394E+01
0.46	4.44	1.44	4.7	0.145E+01	0.54	0.862	.25084E+01
0.61	4.93	1.67	5.4	0.125E+01	0.61	0.760	.31831E+01
0.77	5.38	1.92	6.2	0.109E+01	0.68	0.681	.38992E+01

** CMC HAS BEEN FOUND **

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

0.95	5.80	2.19	7.0	0.966E+00	0.75	0.619	.46454E+01
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Cumulative travel time = 4.6454 sec (0.00 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
0.95	5.80	2.19	7.0	0.966E+00	0.75	.46454E+01

Profile definitions:

BV = Gaussian 1/e (37%) vertical thickness
 BH = Gaussian 1/e (37%) horizontal half-width, normal to trajectory
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic centerline dilution
 C = centerline concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
0.64	5.12	2.94	7.0	0.966E+00	0.02	0.00	2.94	2.92	.46454E+01
0.74	5.32	2.94	7.0	0.966E+00	0.65	0.33	2.94	2.29	.46454E+01
0.83	5.53	2.94	7.0	0.966E+00	0.77	0.46	2.94	2.17	.46454E+01
0.92	5.73	2.94	7.0	0.966E+00	0.85	0.57	2.94	2.09	.46454E+01
1.02	5.94	2.94	7.1	0.959E+00	0.91	0.66	2.94	2.03	.50078E+01
1.11	6.15	2.94	7.3	0.928E+00	0.95	0.73	2.94	1.99	.55514E+01
1.20	6.35	2.94	7.6	0.888E+00	0.98	0.80	2.94	1.96	.60949E+01
1.29	6.56	2.94	7.9	0.854E+00	1.01	0.87	2.94	1.93	.66385E+01
1.39	6.77	2.94	8.2	0.833E+00	1.02	0.93	2.94	1.92	.71821E+01
1.48	6.97	2.94	8.3	0.822E+00	1.03	0.98	2.94	1.91	.77256E+01
1.57	7.18	2.94	8.3	0.814E+00	1.04	1.04	2.94	1.90	.82692E+01

Cumulative travel time = 8.2692 sec (0.00 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

BEGIN MOD155: WEAKLY DEFLECTED SURFACE/BOTTOM PLUME

SURFACE/BOTTOM PLUME into a crossflow

Profile definitions:

BV = Gaussian 1/e (37%) vertical thickness
 BH = Gaussian 1/e (37%) horizontal half-width, normal to trajectory
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic centerline dilution
 C = centerline concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
1.57	7.18	2.94	8.3	0.814E+00	1.04	1.04	2.94	1.90	.82692E+01
2.42	8.03	2.94	12.4	0.547E+00	0.70	2.30	2.94	2.24	.13832E+02
3.31	8.87	2.94	14.9	0.455E+00	0.58	3.33	2.94	2.36	.19807E+02
4.23	9.72	2.94	16.9	0.403E+00	0.51	4.25	2.94	2.43	.26195E+02
5.18	10.57	2.94	18.5	0.367E+00	0.47	5.13	2.94	2.47	.32995E+02
6.17	11.41	2.94	20.0	0.340E+00	0.43	5.97	2.94	2.51	.40208E+02
7.19	12.26	2.94	21.3	0.319E+00	0.41	6.78	2.94	2.53	.47833E+02
8.25	13.11	2.94	22.5	0.302E+00	0.38	7.58	2.94	2.56	.55871E+02
9.34	13.96	2.94	23.6	0.288E+00	0.37	8.37	2.94	2.57	.64321E+02
10.47	14.80	2.94	24.7	0.275E+00	0.35	9.15	2.94	2.59	.73183E+02
11.63	15.65	2.94	25.7	0.264E+00	0.34	9.92	2.94	2.60	.82458E+02

Cumulative travel time = 82.4577 sec (0.02 hrs)

END OF MOD155: WEAKLY DEFLECTED SURFACE/BOTTOM PLUME

BEGIN MOD156: STRONGLY DEFLECTED SURFACE/BOTTOM PLUME

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally from bank/shoreline
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic centerline dilution
 C = centerline concentration (includes reaction effects, if any)
 TT = Cumulative travel time

0.01	0.11	0.32	1.0	0.679E+01	0.11	3.257	.63274E-02
1.75	1.26	0.57	6.2	0.109E+01	0.39	0.199	.15633E+01

** CMC HAS BEEN FOUND **

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

3.97	1.60	0.70	10.0	0.682E+00	0.50	0.104	.37547E+01
6.20	1.81	0.83	12.9	0.525E+00	0.57	0.077	.60399E+01
8.44	1.97	0.97	15.7	0.433E+00	0.63	0.064	.83655E+01
10.68	2.08	1.11	18.3	0.371E+00	0.69	0.055	.10715E+02
12.92	2.18	1.25	20.9	0.324E+00	0.73	0.049	.13082E+02
15.16	2.26	1.39	23.6	0.288E+00	0.78	0.045	.15461E+02
17.40	2.33	1.54	26.3	0.258E+00	0.83	0.041	.17850E+02
19.64	2.38	1.68	29.1	0.234E+00	0.87	0.038	.20248E+02
21.88	2.43	1.83	31.9	0.213E+00	0.91	0.035	.22644E+02

Cumulative travel time = 22.6442 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
21.88	2.43	1.83	31.2	0.218E+00	0.91	.22644E+02

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
20.97	2.42	2.74	31.9	0.213E+00	0.00	0.00	2.74	2.74	.22644E+02
21.24	2.43	2.74	31.9	0.213E+00	1.13	0.57	2.74	1.61	.22644E+02
21.52	2.43	2.74	31.9	0.213E+00	1.34	0.81	2.74	1.40	.22644E+02
21.79	2.43	2.74	31.9	0.213E+00	1.48	0.99	2.74	1.26	.22644E+02
22.07	2.43	2.74	32.8	0.207E+00	1.58	1.14	2.74	1.16	.22847E+02
22.34	2.44	2.74	36.8	0.184E+00	1.65	1.28	2.74	1.09	.23152E+02
22.61	2.44	2.74	42.4	0.160E+00	1.71	1.40	2.74	1.03	.23456E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

22.89	2.44	2.74	47.5	0.143E+00	1.75	1.51	2.74	0.99	.23761E+02
23.16	2.45	2.74	51.0	0.133E+00	1.78	1.61	2.74	0.96	.24065E+02
23.44	2.45	2.74	52.9	0.128E+00	1.80	1.71	2.74	0.94	.24370E+02
23.71	2.45	2.74	54.1	0.125E+00	1.80	1.80	2.74	0.94	.24674E+02

Cumulative travel time = 24.6740 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
23.71	2.45	2.74	54.1	0.125E+00	1.80	1.80	2.74	0.94	.24674E+02
27.41	2.45	2.74	56.0	0.121E+00	1.69	2.00	2.74	1.05	.28788E+02
31.11	2.45	2.74	57.8	0.118E+00	1.60	2.18	2.74	1.14	.32901E+02
34.82	2.45	2.74	59.6	0.114E+00	1.54	2.36	2.74	1.20	.37015E+02
38.52	2.45	2.74	61.5	0.110E+00	1.48	2.53	2.74	1.26	.41128E+02
42.22	2.45	2.74	63.5	0.107E+00	1.44	2.69	2.74	1.30	.45242E+02
45.92	2.45	2.74	65.5	0.104E+00	1.41	2.85	2.74	1.33	.49355E+02
49.63	2.45	2.74	67.7	0.100E+00	1.39	3.01	2.74	1.35	.53469E+02
53.33	2.45	2.74	70.0	0.971E-01	1.37	3.16	2.74	1.37	.57582E+02
57.03	2.45	2.74	72.3	0.939E-01	1.36	3.31	2.74	1.38	.61696E+02
60.73	2.45	2.74	74.9	0.907E-01	1.35	3.45	2.74	1.39	.65809E+02

Cumulative travel time = 65.8091 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.
 Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
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CORMIX SESSION REPORT:

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CORMIX MIXING ZONE EXPERT SYSTEM
CORMIX Version 11.0GTH
HYDROL:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: DEQ Accute Criteria 90th % Velocity
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\Accute90thVelocity.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:42:24

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = bounded
Width BS = 560 m
Channel regularity ICHREG = 1
Ambient flowrate QA = 1814.40 m^3/s
Average depth HA = 3.6 m
Depth at discharge HD = 2.74 m
Darcy-Weisbach friction factor F = 0.0205
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = 2 hours
Instantaneous ambient velocity UA = 0.9 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.45 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter D0 = 0.2032 m
Port cross-sectional area A0 = 0.0324 m^2
Discharge velocity U0 = 3.26 m/s
Discharge flowrate Q0 = 0.105613 m^3/s
Discharge port height H0 = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 0.65 m Lb = 0.02 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.0930 hours Lu = 14.01 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 3.62

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 5600 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 2.74 m

Limiting Dilution S = (QA/Q0)+ 1.0 = 17180.7

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the

discharge design conditions.

Pollutant concentration at NFR edge c = 0.1255 mg/l
Dilution at edge of NFR s = 54.1
NFR Location: x = 23.71 m
(centerline coordinates) y = 2.45 m
z = 2.74 m
NFR plume dimensions: half-width (bh) = 1.80 m
thickness (bv) = 1.80 m
Cumulative travel time: 24.6740 sec.

Buoyancy assessment:

The effluent density is less than the surrounding ambient water density at the discharge level.
Therefore, the effluent is POSITIVELY BUOYANT and will tend to rise towards the surface.

FAR-FIELD MIXING SUMMARY:

Plume becomes vertically fully mixed at 653.74 m downstream.

PLUME BANK CONTACT SUMMARY:

Plume in bounded section contacts one bank only at 60.73 m downstream.

UNSTEADY TIDAL ASSESSMENT:

Because of the unsteadiness of the ambient current during the tidal reversal, CORMIX predictions have been TERMINATED at:

x = 1031.32 m
y = -1 m
z = 2.74 m.

For this condition AFTER TIDAL REVERSAL, mixed water from the previous half-cycle becomes re-entrained into the near field of the discharge, increasing pollutant concentrations compared to steady-state predictions. A pool of mixed water formed at slack tide will be advected downstream in this phase.

***** TOXIC DILUTION ZONE SUMMARY *****

Recall: The TDZ corresponds to the three (3) criteria issued in the USEPA Technical Support Document (TSD) for Water Quality-based Toxics Control, 1991 (EPA/505/2-90-001).

Criterion maximum concentration (CMC) = 1.04 mg/l
Corresponding dilution = 6.528846
The CMC was encountered at the following plume position:
Plume location: x = 1.90 m
(centerline coordinates) y = 1.29 m
z = 0.58 m
Plume dimension: half-width (bh) = 0.03 m
thickness (bv) = 0.03 m

Computed distance from port opening to CMC location = 2.31 m.

CRITERION 1: This location is within 50 times the discharge length scale of Lq = 0.18 m.

+++++ The discharge length scale TEST for the TDZ has been SATISFIED. +++++

Computed horizontal distance from port opening to CMC location = 2.29 m.

CRITERION 2: This location is within 5 times the ambient water depth of HD = 2.74 m.

+++++ The ambient depth TEST for the TDZ has been SATISFIED. +++++

CRITERION 3: No RMZ has been defined. Therefore, the Regulatory Mixing zone test for the TDZ cannot be applied.

The diffuser discharge velocity is equal to 3.26 m/s.
This exceeds the value of 3.0 m/s recommended in the TSD.

*** All three CMC criteria for the TDZ are SATISFIED for this discharge. ***
***** REGULATORY MIXING ZONE SUMMARY *****
No RMZ has been specified.

However:

The CCC was encountered at the following plume position:
The CCC for the toxic pollutant was encountered at the following plume position:

plume position:
CCC = 0.16 mg/l
Corresponding dilution = 42.4
Plume location: x = 22.62 m
(centerline coordinates) y = 2.44 m
z = 2.74 m

Computed horizontal distance from port opening to CCC location = 22.88

Plume dimensions: half-width (bh) = 1.40 m
thickness (bv) = 1.71 m

***** FINAL DESIGN ADVICE AND COMMENTS *****
REMINDER: The user must take note that HYDRODYNAMIC MODELING by any known technique is NOT AN EXACT SCIENCE.

Extensive comparison with field and laboratory data has shown that the CORMIX predictions on dilutions and concentrations (with associated plume geometries) are reliable for the majority of cases and are accurate to within about +/-50% (standard deviation).

As a further safeguard, CORMIX will not give predictions whenever it judges the design configuration as highly complex and uncertain for prediction.

1.22 2.22 0.77 5.6 0.122E+01 0.46 0.429 .15043E+01
**** CMC HAS BEEN FOUND ****
 The pollutant concentration in the plume falls below CMC value of 0.104E+01
 in the current prediction interval.
 This is the extent of the TOXIC DILUTION ZONE.

2.28	2.63	0.92	8.6	0.791E+00	0.59	0.237	.29921E+01
3.35	2.91	1.06	11.2	0.609E+00	0.69	0.170	.46374E+01
4.44	3.12	1.19	13.5	0.502E+00	0.76	0.137	.63853E+01
5.56	3.29	1.33	15.8	0.429E+00	0.83	0.116	.82454E+01
6.66	3.42	1.47	18.0	0.377E+00	0.89	0.103	.10102E+02
7.80	3.54	1.61	20.2	0.336E+00	0.94	0.093	.12042E+02
8.90	3.64	1.75	22.4	0.304E+00	1.00	0.086	.13958E+02
10.00	3.73	1.89	24.5	0.277E+00	1.05	0.080	.15894E+02

Cumulative travel time = 15.8941 sec (0.00 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
10.00	3.73	1.89	24.4	0.279E+00	1.05	.15894E+02

Profile definitions:
 BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
8.96	3.69	2.94	24.5	0.277E+00	0.00	0.00	2.94	2.94	.15894E+02
9.27	3.71	2.94	24.5	0.277E+00	1.32	0.67	2.94	1.62	.15894E+02
9.59	3.72	2.94	24.5	0.277E+00	1.57	0.94	2.94	1.37	.15894E+02
9.90	3.73	2.94	24.5	0.277E+00	1.73	1.15	2.94	1.21	.15894E+02
10.22	3.74	2.94	25.2	0.269E+00	1.84	1.33	2.94	1.10	.16316E+02
10.53	3.75	2.94	28.3	0.240E+00	1.93	1.49	2.94	1.01	.16943E+02
10.84	3.76	2.94	32.6	0.208E+00	2.00	1.63	2.94	0.94	.17570E+02
11.16	3.78	2.94	36.6	0.186E+00	2.05	1.76	2.94	0.89	.18197E+02
11.47	3.79	2.94	39.2	0.173E+00	2.08	1.88	2.94	0.86	.18824E+02
11.78	3.80	2.94	40.7	0.167E+00	2.10	2.00	2.94	0.84	.19451E+02
12.10	3.81	2.94	41.6	0.163E+00	2.11	2.11	2.94	0.83	.20078E+02

Cumulative travel time = 20.0782 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

**** End of NEAR-FIELD REGION (NFR) ****

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:
 BV = top-hat thickness, measured vertically
 BH = top-hat half-width, measured horizontally in Y-direction
 ZU = upper plume boundary (Z-coordinate)
 ZL = lower plume boundary (Z-coordinate)
 S = hydrodynamic average (bulk) dilution
 C = average (bulk) concentration (includes reaction effects, if any)
 TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
12.10	3.81	2.94	41.6	0.163E+00	2.11	2.11	2.94	0.83	.20078E+02

****WATER QUALITY STANDARD OR CCC HAS BEEN FOUND****
 The pollutant concentration in the plume falls below water quality standard
 or CCC value of 0.160E+00 in the current prediction interval.
 This is the spatial extent of concentrations exceeding the water quality
 standard or CCC value.

14.98	3.81	2.94	43.2	0.157E+00	1.89	2.44	2.94	1.05	.25845E+02
17.86	3.81	2.94	44.5	0.153E+00	1.73	2.75	2.94	1.21	.31612E+02
20.75	3.81	2.94	45.7	0.149E+00	1.61	3.04	2.94	1.33	.37379E+02
23.63	3.81	2.94	46.7	0.145E+00	1.51	3.32	2.94	1.43	.43146E+02
26.51	3.81	2.94	47.7	0.142E+00	1.43	3.59	2.94	1.51	.48914E+02
29.40	3.81	2.94	48.7	0.140E+00	1.36	3.85	2.94	1.58	.54681E+02
32.28	3.81	2.94	49.5	0.137E+00	1.30	4.10	2.94	1.64	.60448E+02
35.16	3.81	2.94	50.4	0.135E+00	1.25	4.35	2.94	1.69	.66215E+02
38.05	3.81	2.94	51.2	0.133E+00	1.21	4.58	2.94	1.73	.71982E+02
40.93	3.81	2.94	52.1	0.130E+00	1.17	4.81	2.94	1.77	.77749E+02

Cumulative travel time = 77.7488 sec (0.02 hrs)

Plume is ATTACHED to RIGHT bank/shore.
 Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
40.93	-1.00	2.94	52.1	0.130E+00	1.17	9.63	2.94	1.77	.77749E+02
95.18	-1.00	2.94	63.0	0.108E+00	1.04	13.53	2.94	1.90	.18624E+03

CORMIX SESSION REPORT:

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CORMIX MIXING ZONE EXPERT SYSTEM

CORMIX Version 11.0GTH

HYDRO1:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: DEQ Chronic Criteria 50th & Velocity (Ebb)
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\Chronic50thVelocityEbb.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:43:45

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 2.94 m
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = 2.1 hours
Instantaneous ambient velocity UA = 0.5 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.2381 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter D0 = 0.2032 m
Port cross-sectional area A0 = 0.0324 m^2
Discharge velocity U0 = 3.26 m/s
Discharge flowrate Q0 = 0.105613 m^3/s
Discharge port height H0 = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration C0 = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 1.17 m Lb = 0.12 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.1422 hours Lu = 17.32 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 6.51

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 5600 m downstream

HYDRODYNAMIC CLASSIFICATION:

FLOW CLASS = H2
This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 2.94 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge c = 0.163 mg/l
Dilution at edge of NFR s = 41.6
NFR Location: x = 12.10 m
(centerline coordinates) y = 3.81 m
z = 2.94 m

** CMC HAS BEEN FOUND **

The pollutant concentration in the plume falls below CMC value of 0.104E+01 in the current prediction interval.

This is the extent of the TOXIC DILUTION ZONE.

3.26	2.89	1.05	11.0	0.616E+00	0.68	0.173	.45394E+01
5.44	3.27	1.31	15.7	0.434E+00	0.82	0.118	.80807E+01
7.64	3.53	1.59	20.0	0.339E+00	0.94	0.094	.11810E+02
9.79	3.72	1.86	24.3	0.280E+00	1.04	0.081	.15564E+02
12.00	3.87	2.15	28.7	0.236E+00	1.13	0.071	.19471E+02
14.20	3.99	2.43	33.3	0.204E+00	1.22	0.064	.23432E+02
16.41	4.09	2.71	38.0	0.179E+00	1.31	0.059	.27435E+02

WATER QUALITY STANDARD OR CCC HAS BEEN FOUND

The pollutant concentration in the plume falls below water quality standard or CCC value of 0.160E+00 in the current prediction interval.

This is the spatial extent of concentrations exceeding the water quality standard or CCC value.

18.62	4.17	2.99	42.9	0.158E+00	1.39	0.054	.31472E+02
20.78	4.24	3.26	47.8	0.142E+00	1.47	0.050	.35433E+02

Cumulative travel time = 35.4333 sec (0.01 hrs)

END OF CORJET (MOD110): JET/PLUME NEAR-FIELD MIXING REGION

BEGIN MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

Control volume inflow:

X	Y	Z	S	C	B	TT
20.78	4.24	3.26	47.6	0.143E+00	1.47	.35433E+02

Profile definitions:

- BV = top-hat thickness, measured vertically
- BH = top-hat half-width, measured horizontally in Y-direction
- ZU = upper plume boundary (Z-coordinate)
- ZL = lower plume boundary (Z-coordinate)
- S = hydrodynamic average (bulk) dilution
- C = average (bulk) concentration (includes reaction effects, if any)
- TT = Cumulative travel time

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
19.31	4.21	4.74	47.8	0.142E+00	0.00	0.00	4.74	4.74	.35433E+02
19.75	4.22	4.74	47.8	0.142E+00	1.85	0.93	4.74	2.89	.35433E+02
20.19	4.23	4.74	47.8	0.142E+00	2.19	1.31	4.74	2.55	.35433E+02
20.63	4.23	4.74	47.8	0.142E+00	2.41	1.61	4.74	2.33	.35433E+02
21.08	4.24	4.74	49.1	0.138E+00	2.57	1.86	4.74	2.17	.36023E+02
21.52	4.25	4.74	55.2	0.123E+00	2.69	2.08	4.74	2.05	.36907E+02
21.96	4.25	4.74	63.6	0.107E+00	2.78	2.27	4.74	1.96	.37790E+02
22.40	4.26	4.74	71.3	0.952E-01	2.85	2.46	4.74	1.89	.38674E+02
22.84	4.27	4.74	76.6	0.887E-01	2.90	2.63	4.74	1.84	.39558E+02
23.28	4.27	4.74	79.4	0.855E-01	2.93	2.79	4.74	1.81	.40441E+02
23.73	4.28	4.74	81.3	0.836E-01	2.94	2.94	4.74	1.80	.41325E+02

Cumulative travel time = 41.3249 sec (0.01 hrs)

END OF MOD131: LAYER BOUNDARY/TERMINAL LAYER APPROACH

** End of NEAR-FIELD REGION (NFR) **

BEGIN MOD141: BUOYANT AMBIENT SPREADING

Profile definitions:

- BV = top-hat thickness, measured vertically
- BH = top-hat half-width, measured horizontally in Y-direction
- ZU = upper plume boundary (Z-coordinate)
- ZL = lower plume boundary (Z-coordinate)
- S = hydrodynamic average (bulk) dilution
- C = average (bulk) concentration (includes reaction effects, if any)
- TT = Cumulative travel time

Plume Stage 1 (not bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
23.73	4.28	4.74	81.3	0.836E-01	2.94	2.94	4.74	1.80	.41325E+02
26.46	4.28	4.74	83.2	0.817E-01	2.75	3.21	4.74	1.99	.46786E+02
29.19	4.28	4.74	84.9	0.800E-01	2.60	3.47	4.74	2.14	.52247E+02
31.92	4.28	4.74	86.5	0.785E-01	2.47	3.72	4.74	2.27	.57708E+02
34.65	4.28	4.74	88.0	0.772E-01	2.36	3.96	4.74	2.38	.63169E+02
37.38	4.28	4.74	89.4	0.759E-01	2.27	4.19	4.74	2.47	.68630E+02
40.11	4.28	4.74	90.8	0.748E-01	2.19	4.42	4.74	2.55	.74091E+02
42.84	4.28	4.74	92.2	0.737E-01	2.11	4.64	4.74	2.63	.79552E+02
45.57	4.28	4.74	93.5	0.727E-01	2.05	4.86	4.74	2.69	.85013E+02
48.30	4.28	4.74	94.7	0.717E-01	1.99	5.07	4.74	2.75	.90474E+02
51.03	4.28	4.74	96.0	0.708E-01	1.94	5.28	4.74	2.80	.95935E+02

Cumulative travel time = 95.9350 sec (0.03 hrs)

Plume is ATTACHED to RIGHT bank/shore.

Plume width is now determined from RIGHT bank/shore.

Plume Stage 2 (bank attached):

X	Y	Z	S	C	BV	BH	ZU	ZL	TT
51.03	-1.00	4.74	96.0	0.708E-01	1.94	10.56	4.74	2.80	.95935E+02
124.99	-1.00	4.74	119.3	0.569E-01	1.65	15.58	4.74	3.09	.24386E+03

CORMIX SESSION REPORT:

XX

CORMIX MIXING ZONE EXPERT SYSTEM

CORMIX Version 11.0GTH

HYDRO1:Version-11.0.0.0 April,2018

SITE NAME/LABEL: Bay City Outfall
DESIGN CASE: DEQ Chronic Criteria 50% Velocity (Flood)
FILE NAME: U:\rstillmaker\2018\BayCityMixZn\Output\Chronic50thVelocityFlood.prd
Using subsystem CORMIX1: Single Port Discharges
Start of session: 09/06/2018--16:45:12

SUMMARY OF INPUT DATA:

AMBIENT PARAMETERS:

Cross-section = unbounded
Average depth HA = 4.11 m
Depth at discharge HD = 4.74 m
Darcy-Weisbach friction factor F = 0.0196
Calculated from Manning's n = 0.02
Wind velocity UW = 5.7 m/s
TIDAL SIMULATION at time Tsim = -1 hours
Instantaneous ambient velocity UA = 0.5 m/s
Maximum tidal velocity UaMAX = 1 m/s
Rate of tidal reversal dUA/dt = 0.5 (m/s)/hour
Period of reversal T = 12.4 hours
Stratification Type STRCND = U
Surface density RHOAS = 1012.74 kg/m^3
Bottom density RHOAB = 1012.74 kg/m^3

DISCHARGE PARAMETERS:

Single Port Discharge
Nearest bank = right
Distance to bank DISTB = 1 m
Port diameter DO = 0.2032 m
Port cross-sectional area AO = 0.0324 m^2
Discharge velocity UO = 3.26 m/s
Discharge flowrate QO = 0.105613 m^3/s
Discharge port height HO = 0.30 m
Vertical discharge angle THETA = 10 deg
Horizontal discharge angle SIGMA = 90 deg
Discharge density RHO0 = 998 kg/m^3
Density difference DRHO = 14.7400 kg/m^3
Buoyant acceleration GPO = 0.1427 m/s^2
Discharge concentration CO = 6.79 mg/l
Surface heat exchange coeff. KS = 0 m/s
Coefficient of decay KD = 0 /s

DISCHARGE/ENVIRONMENT LENGTH SCALES:

LQ = 0.18 m Lm = 1.17 m Lb = 0.12 m
LM = 3.66 m Lm' = 99999 m Lb' = 99999 m

UNSTEADY TIDAL SCALES:

Tu = 0.0867 hours Lu = 13.53 m Lmin= 0.18 m

NON-DIMENSIONAL PARAMETERS:

Port densimetric Froude number FRO = 19.12
Velocity ratio R = 6.51

MIXING ZONE / TOXIC DILUTION ZONE / AREA OF INTEREST PARAMETERS:

Toxic discharge = yes
CMC concentration CMC = 1.04 mg/l
CCC concentration CCC = 0.16 mg/l
Water quality standard specified = given by CCC value
Regulatory mixing zone = no
Region of interest = 5600 m downstream

HYDRODYNAMIC CLASSIFICATION:

| FLOW CLASS = H2 |

This flow configuration applies to a layer corresponding to the full water depth at the discharge site.
Applicable layer depth = water depth = 4.74 m

MIXING ZONE EVALUATION (hydrodynamic and regulatory summary):

X-Y-Z Coordinate system:

Origin is located at the BOTTOM below the port/diffuser center:
1 m from the right bank/shore.
Number of display steps NSTEP = 10 per module.

NEAR-FIELD REGION (NFR) CONDITIONS :

Note: The NFR is the zone of strong initial mixing. It has no regulatory implication. However, this information may be useful for the discharge designer because the mixing in the NFR is usually sensitive to the discharge design conditions.

Pollutant concentration at NFR edge c = 0.0836 mg/l
Dilution at edge of NFR s = 81.3
NFR Location: x = 23.73 m
(centerline coordinates) y = 4.28 m
z = 4.74 m

SHN Geotechnical Drilling Work Scope – Bandon, OR

Subsurface Investigation

SHN is to retain the services of an Oregon licensed C-57 drilling subcontractor to advance one (1) geotechnical boring to a depth of approximately 100 feet or refusal (whichever is shallowest) with a truck-mounted rotary wash drilling rig. Mud rotary drilling methods are required in anticipation of the presence of fully saturated loose historic fill materials and soft fine-grained sediments underlying the fill materials.

Borehole advancement by mud rotary drilling is achieved by rapid rotation of a drill bit which is mounted at the end of drill pipe. In this method, mud is pumped down the drill pipe and out through the ports in the drill bit. The drill bit cuts the formation into small pieces, called cuttings, which are removed by pumping drilling fluid, called mud, through the drill pipe, out the drill bit and up the annulus between the borehole and drill pipe. The drilling fluid is also used to cool the drill bit and stabilize the borehole wall and prevent fluid loss into the formation. The drilling fluid then flows up the annular space between the borehole walls and the drill pipe carrying cuttings in suspension to the surface. The cuttings are then discharged onto a metal screen placed over the mud tub for viewing, sampling, and logging.

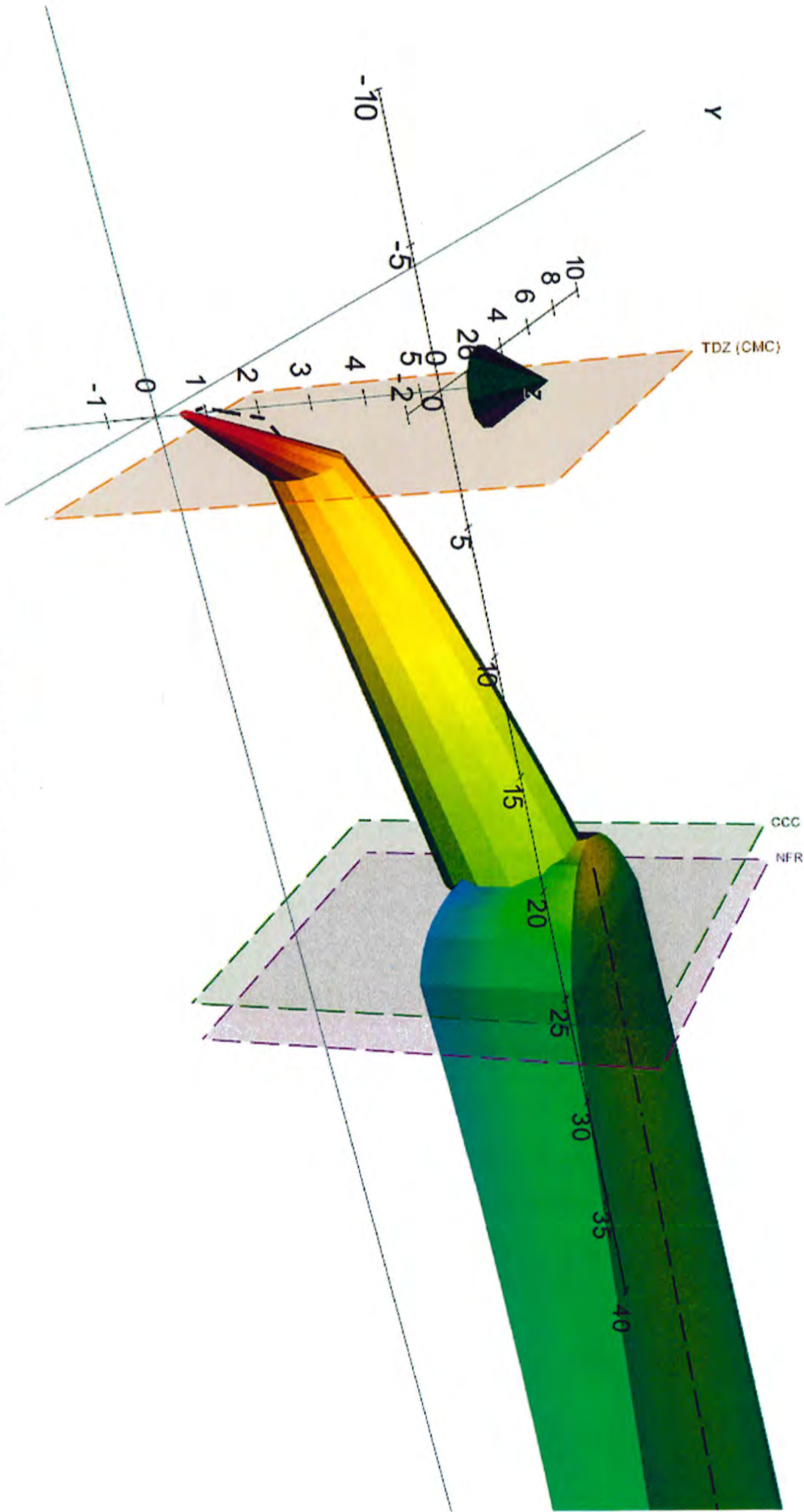
The boring will be centrally located in the proposed building footprint to delineate the thickness of historic fill materials and determine the depth suitably dense foundation load bearing materials. The boring will be closed with Portland cement grout slurry from the bottom of the borehole to the ground surface using the tremie method. All drilling fluid and drill cuttings will be contained in 55-gallon drums, and left on-site. Disposal of the drummed materials will be left to the discretion of the Project Owner.

SHN will supervise the drilling of the geotechnical boring, perform laboratory testing on selected samples to characterize material strength and index properties. The soil and laboratory data will be utilized to develop recommendations for ground improvements and deep foundation support of the proposed structure.

(c)ess (mg/l)
 0.422 1.077 2.746 7.000

a M-H-W max eb current
 Flow Class: H2
 CORMIX1 Simulation
 Distortion Scale: Y:X = 1.4 Z:X = 2.2
 Visualization up to X = 988 m (out of ROI X = 988 m)

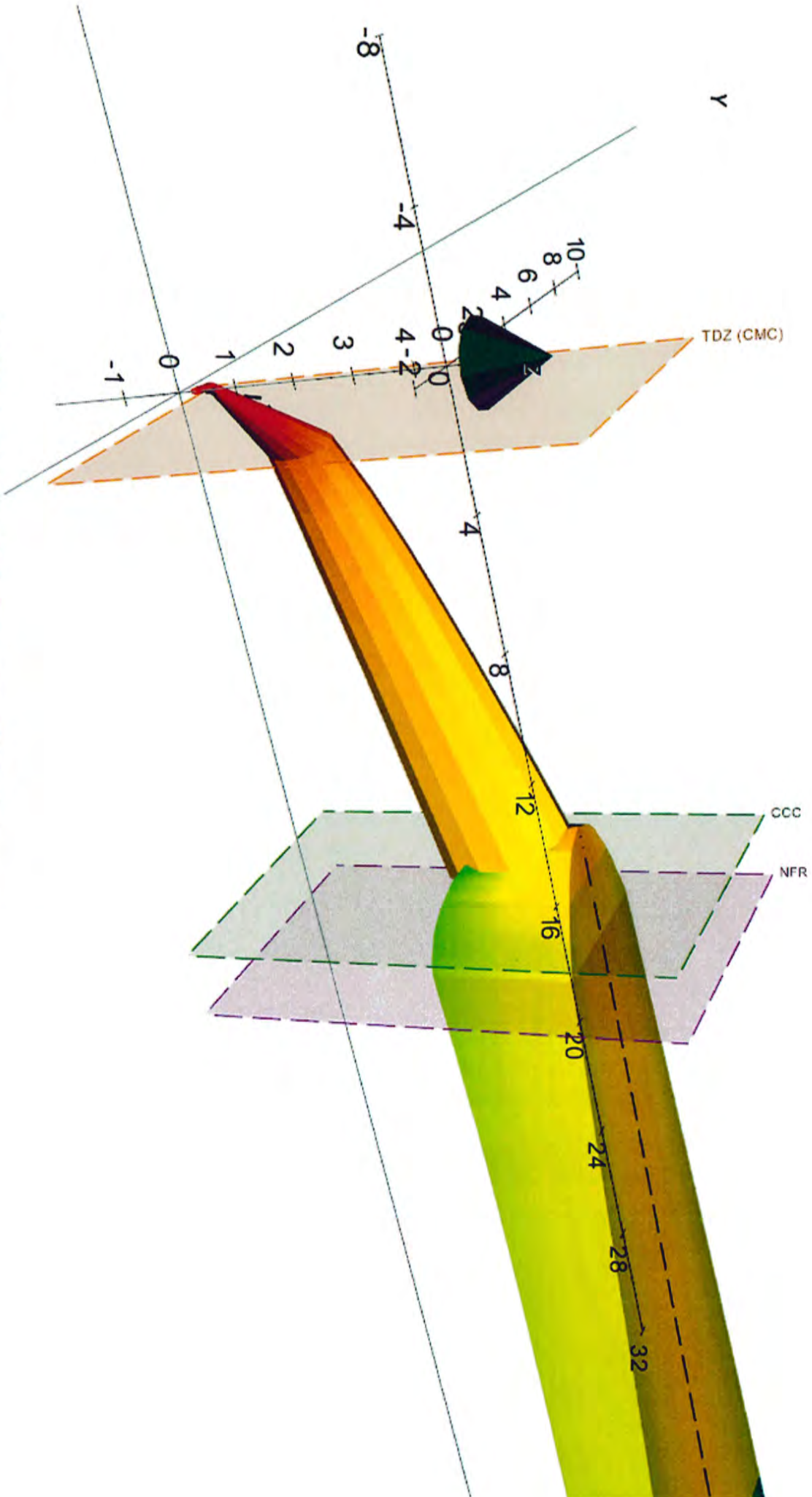
Plume Centerline
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)

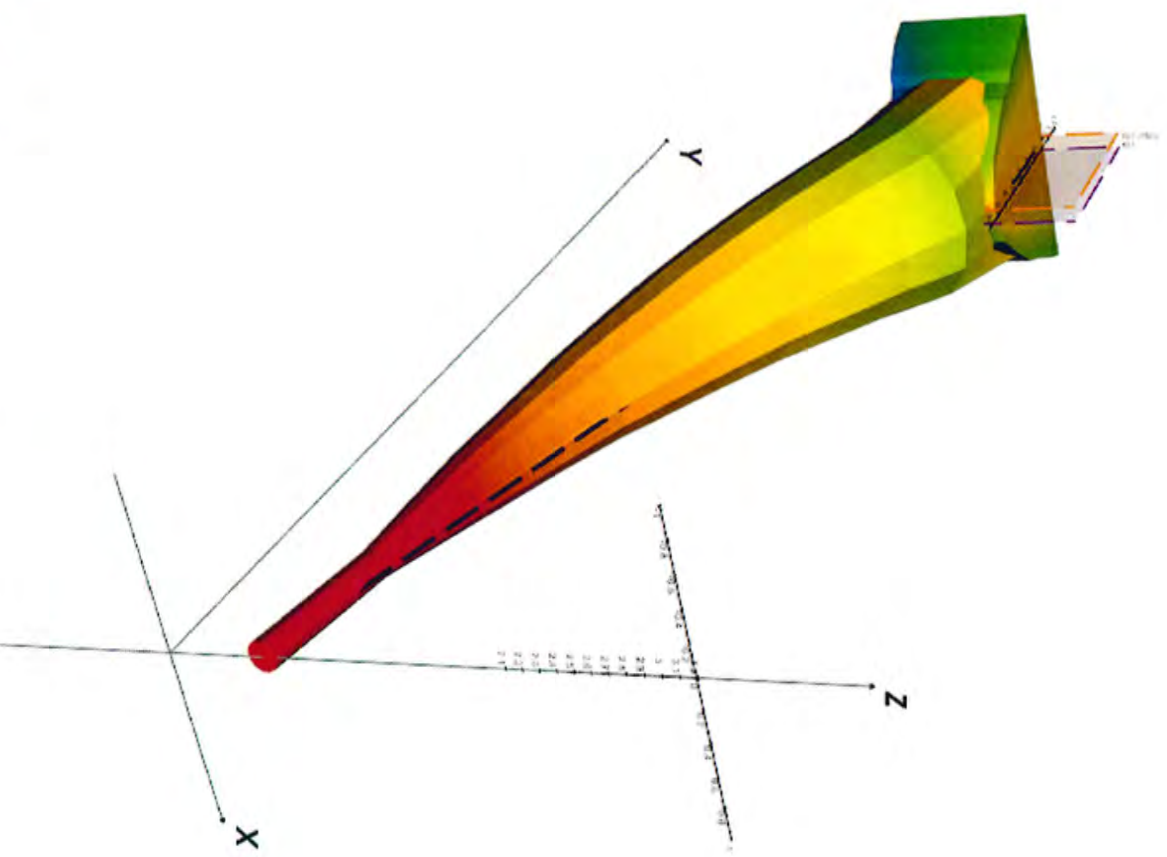
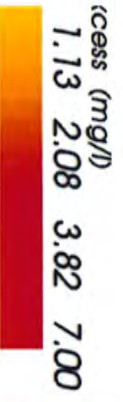


Concentration (mg/l)
 0.1575 0.5578 1.9761 7.0000

b mid High and Low Slack single diff 8...
 Flow Class: H2
 Origin: Ambient Bottom
 CORMIX1 Simulation
 Length units in meters
 Distortion Scale: Y:X = 1 Z:X = 1.8
 Visualization up to X = 870 m (out of ROI X = 870 m)

Plume Centerline
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)





c Low Stack single diff 8 inch port

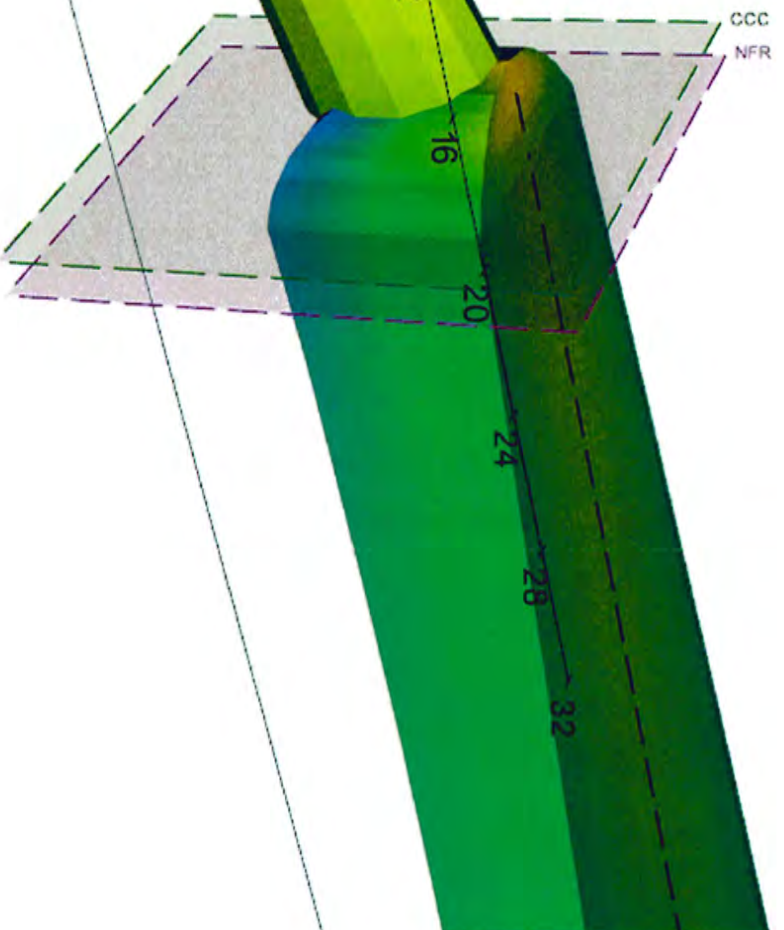
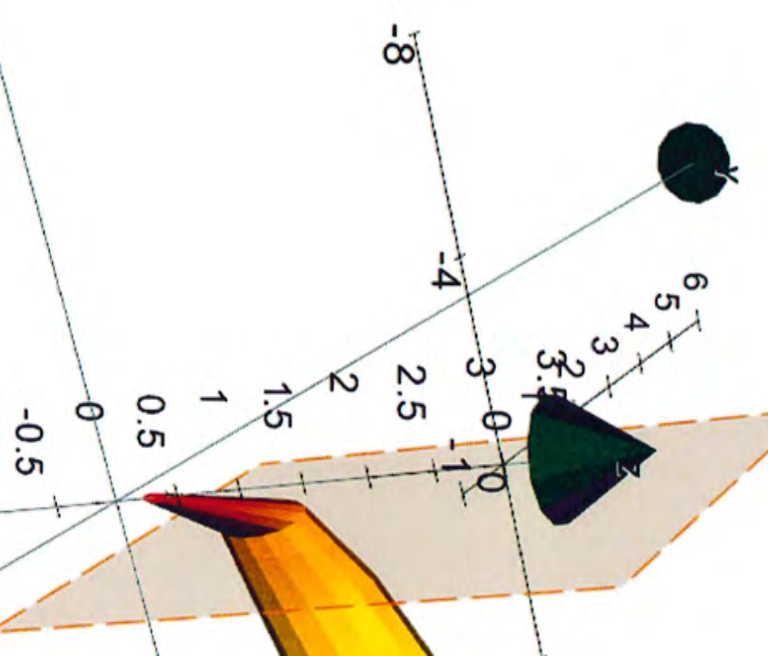
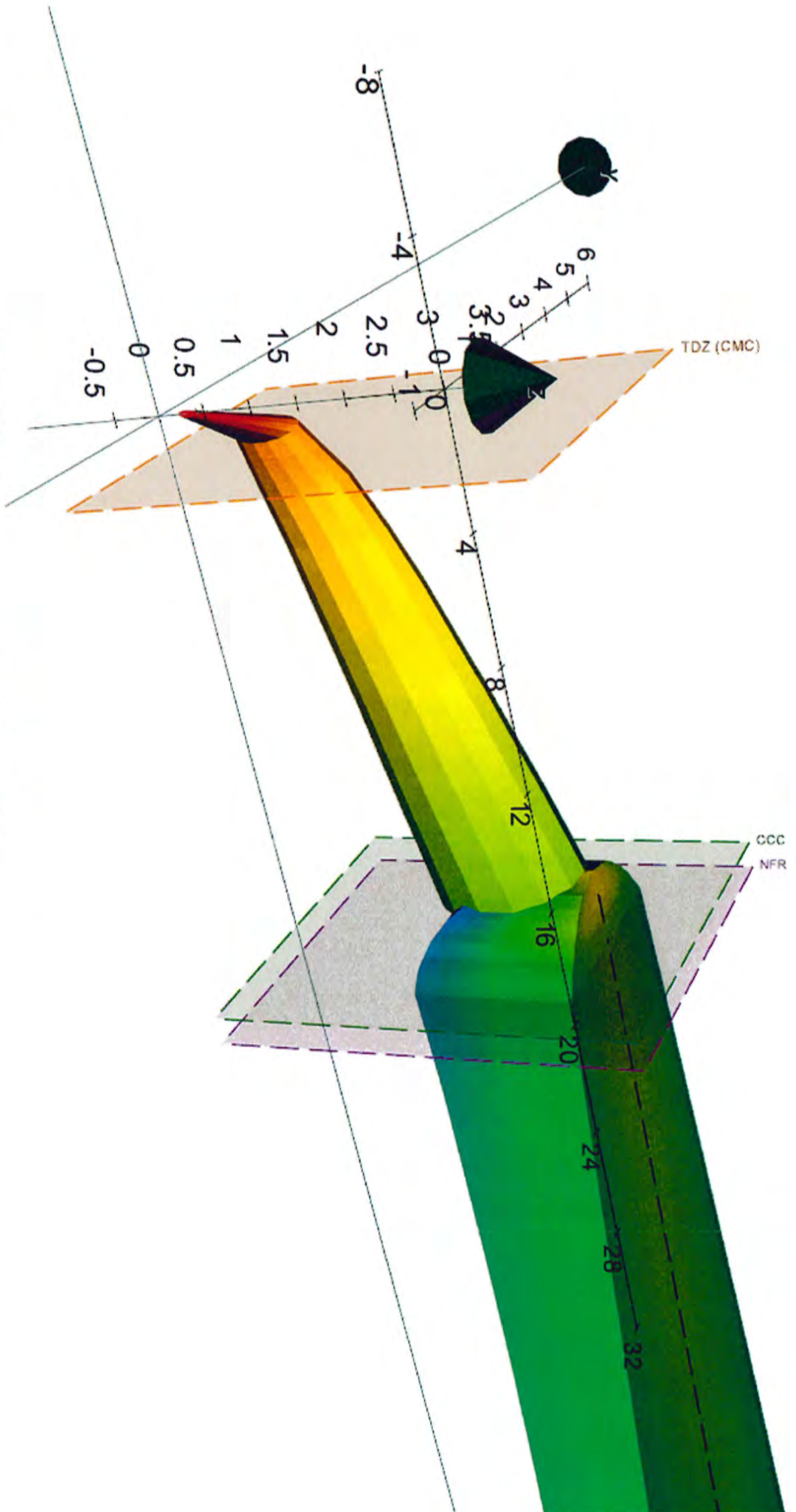
Flow Class: H4-90 Origin: Ambient Bottom
 CORRIMIX1 Simulation Length units in meters
 Distortion Scale: Y:X = 1 ZX = 1
 Visualization up to X = 0.00 m (out of ROI X = 0 m)

- Plume Centerline
- Toxic Dilution Zone (TDZ- CMC)
- End of Near Field Region (NFR)
- Cormix Module Boundary (MOD)

(Cess (mg/l))
 0.422 1.077 2.746 7.000

D MILLW 8 inch port
 Flow Class: H2
 CORMIX1 Simulation
 Distortion Scale: Y:X = 1.9 Z:X = 3
 Visualization up to X = 917 m (out of ROI X = 917 m)

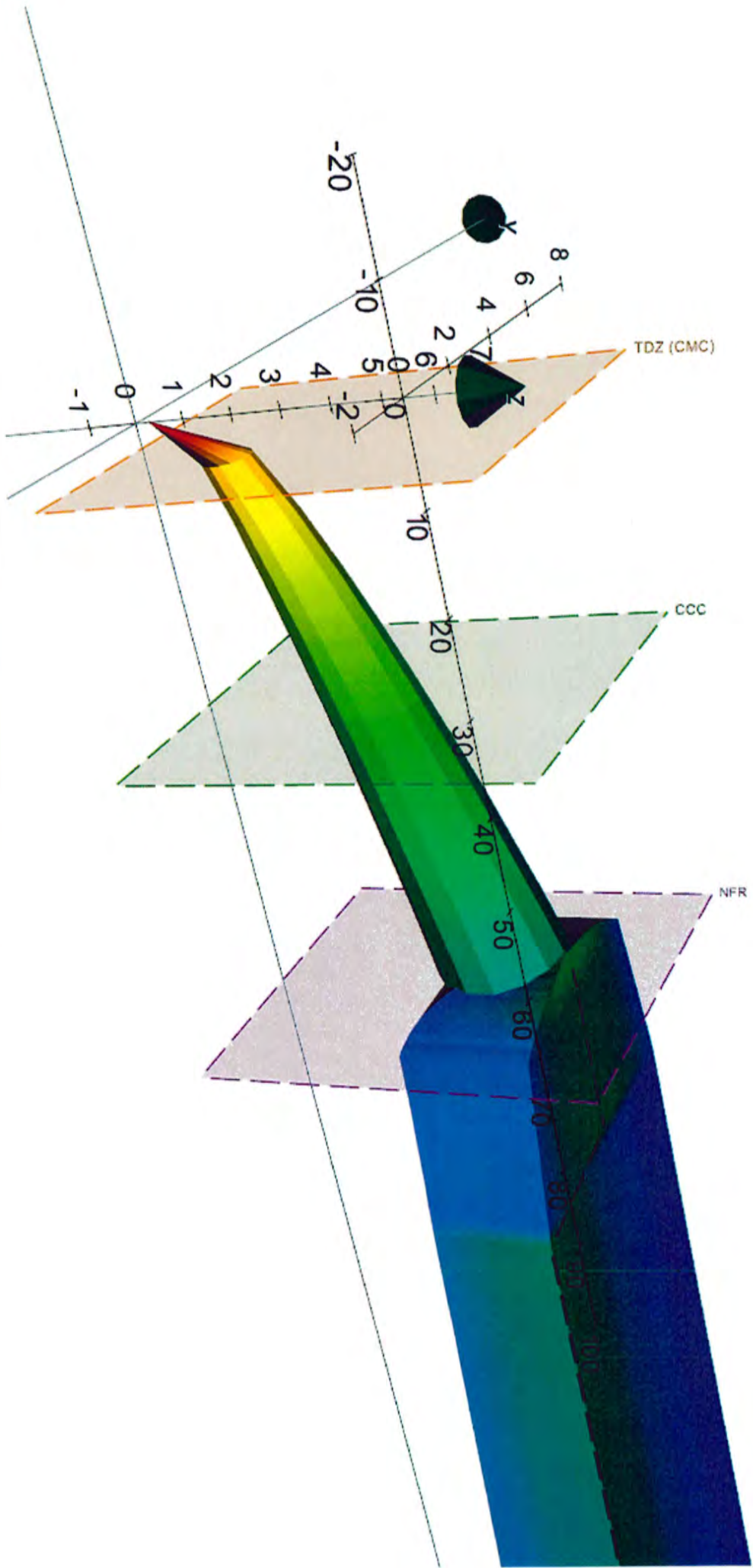
Plume Centerline
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)



(c) (mg/l)
 0.422 1.077 2.746 7.000

e MHIW max flood current 8 inch port
 Flow Class: H2
 Origin: Ambient Bottom
 CORMIX1 Simulation
 Length units in meters
 Distortion Scale: YX = 5 ZX = 5
 Visualization up to X = 1719 m (out of ROI X = 1719 m)

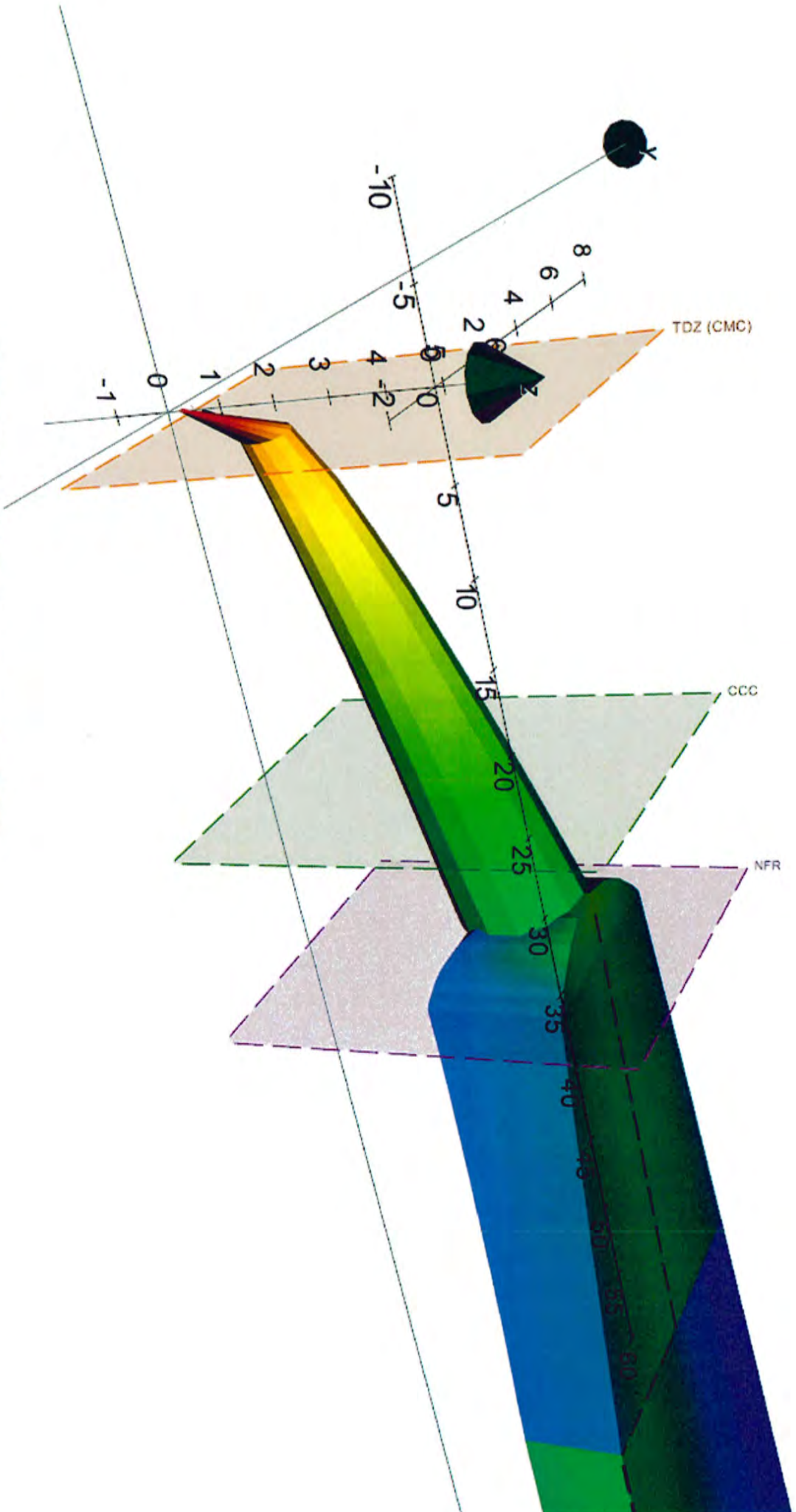
--- Plume Centerline
 --- Toxic Dilution Zone (TDZ - CMC)
 --- Water Quality Standard (WQS - CCC)
 --- End of Near Field Region (NFR)
 --- Cormix Module Boundary (MOD)

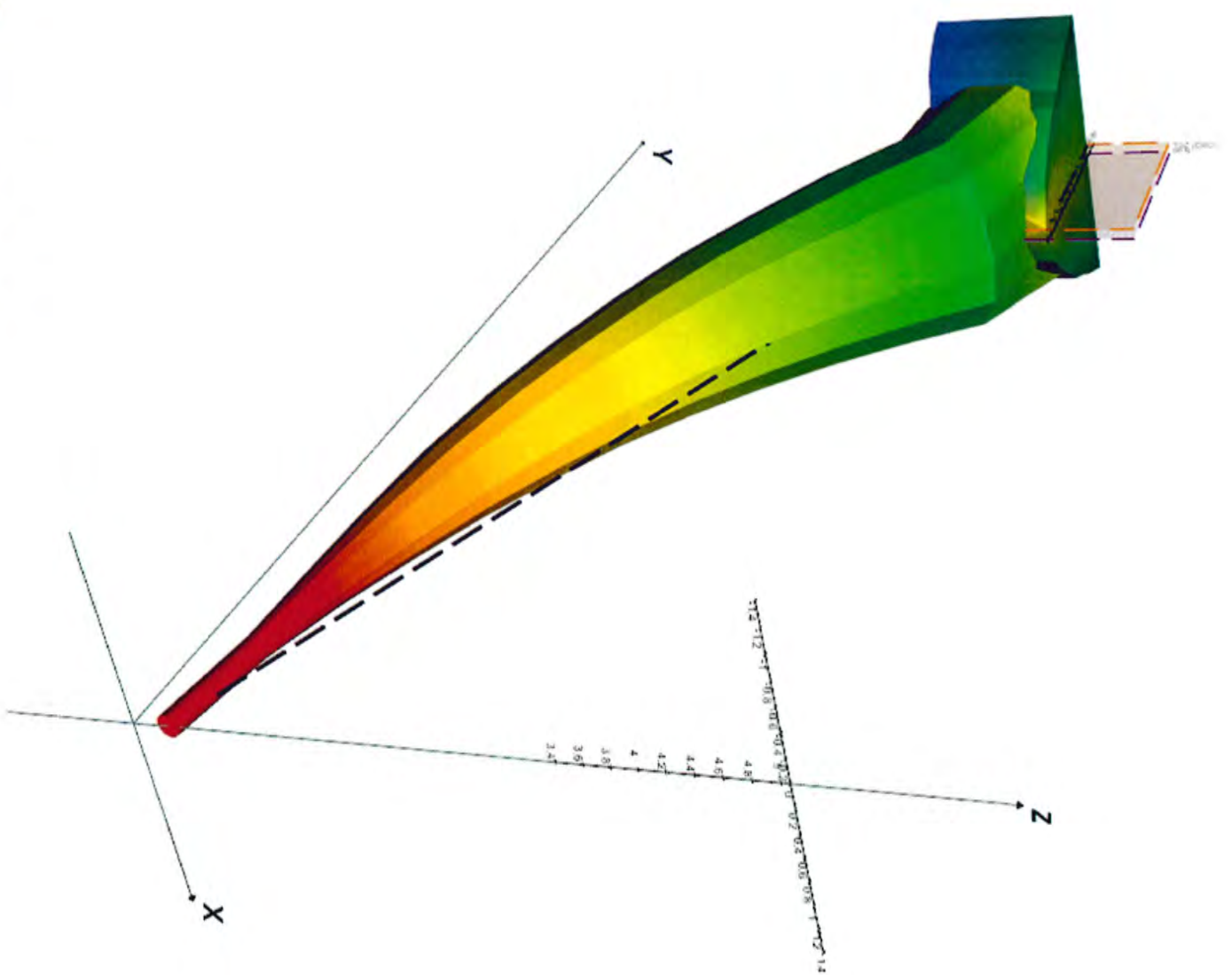
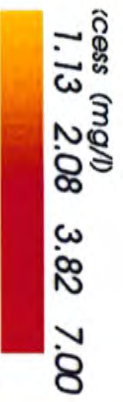


Concentration (mg/l)
 0.422 1.077 2.746 7.000

MidMLLW Floodion-H-HW8inch port
 Flow Class: H2
 Origin: Ambient Bottom
 CORMIX1 Simulation
 Length units in meters
 Distortion Scale: Y/X = 2.7 Z/X = 3.2
 Visualization up to X = 1382 m (out of ROI X = 1382 m)

Plume Centerline
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)





g HighWaterStackInchport

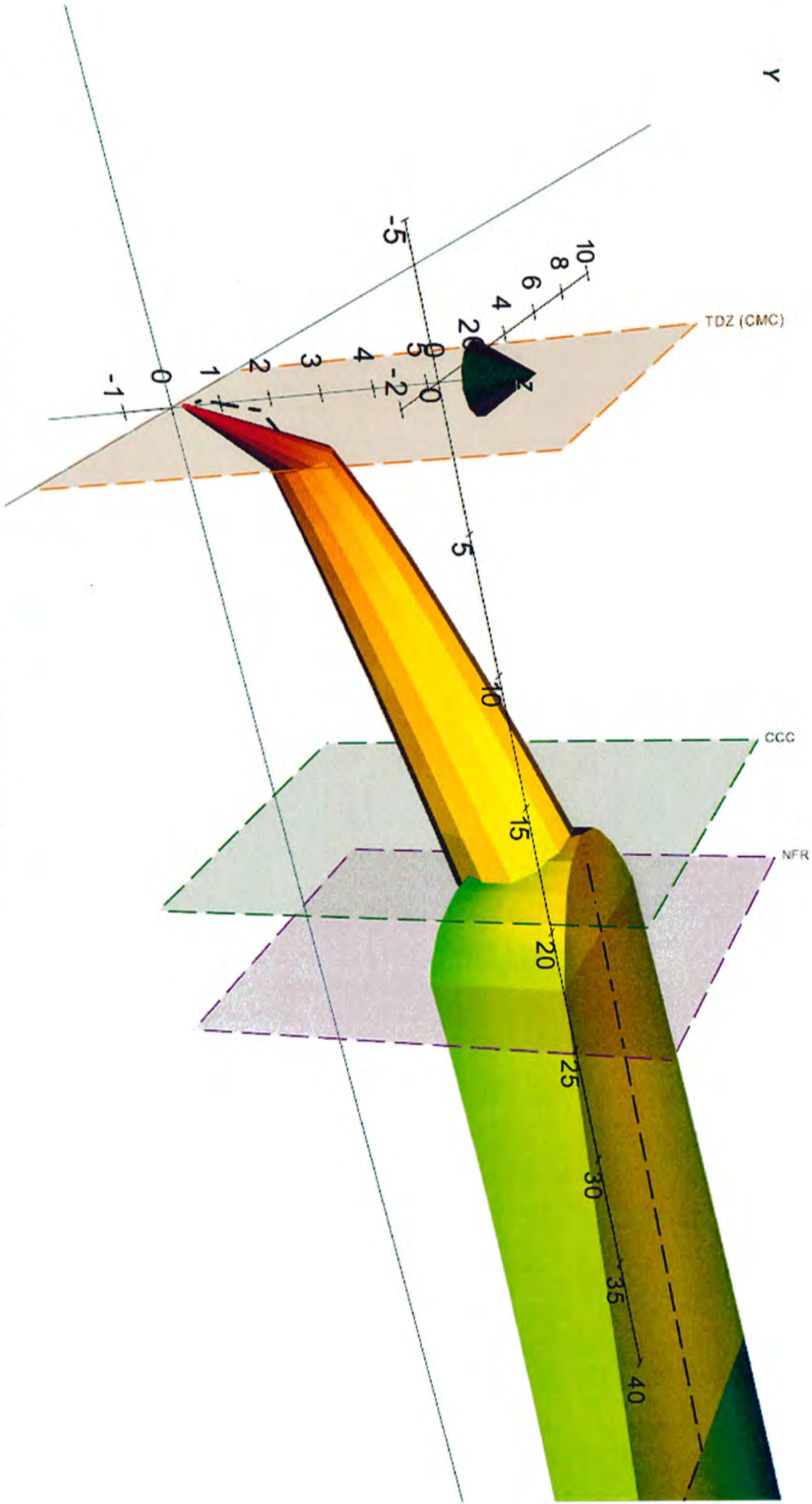
Flow Class: H4-90 Origin: Ambient Bottom
 CORMIX1 Simulation Length units in meters
 Distortion Scale: Y:X = 1 Z:X = 1
 Visualization up to X = 0.00 m (out of ROI X = 0 m)

- Plume Centerline
- Toxic Dilation Zone (TDZ - CMC)
- End of Near Field Region (NFR)
- Cornix Module Boundary (MOD)

H₂ (mg/l)
 0.1575 0.5578 1.9761 7.0000

ChronicSO₂H Velocity Flood
 Flow Class: H2
 Origin: Ambient Bottom
 CORMIX1 Simulation
 Length units in meters
 Distortion Scale: Y:X = 1.4 Z:X = 1.9
 Visualization up to X = 794 m (out of ROI X = 794 m)

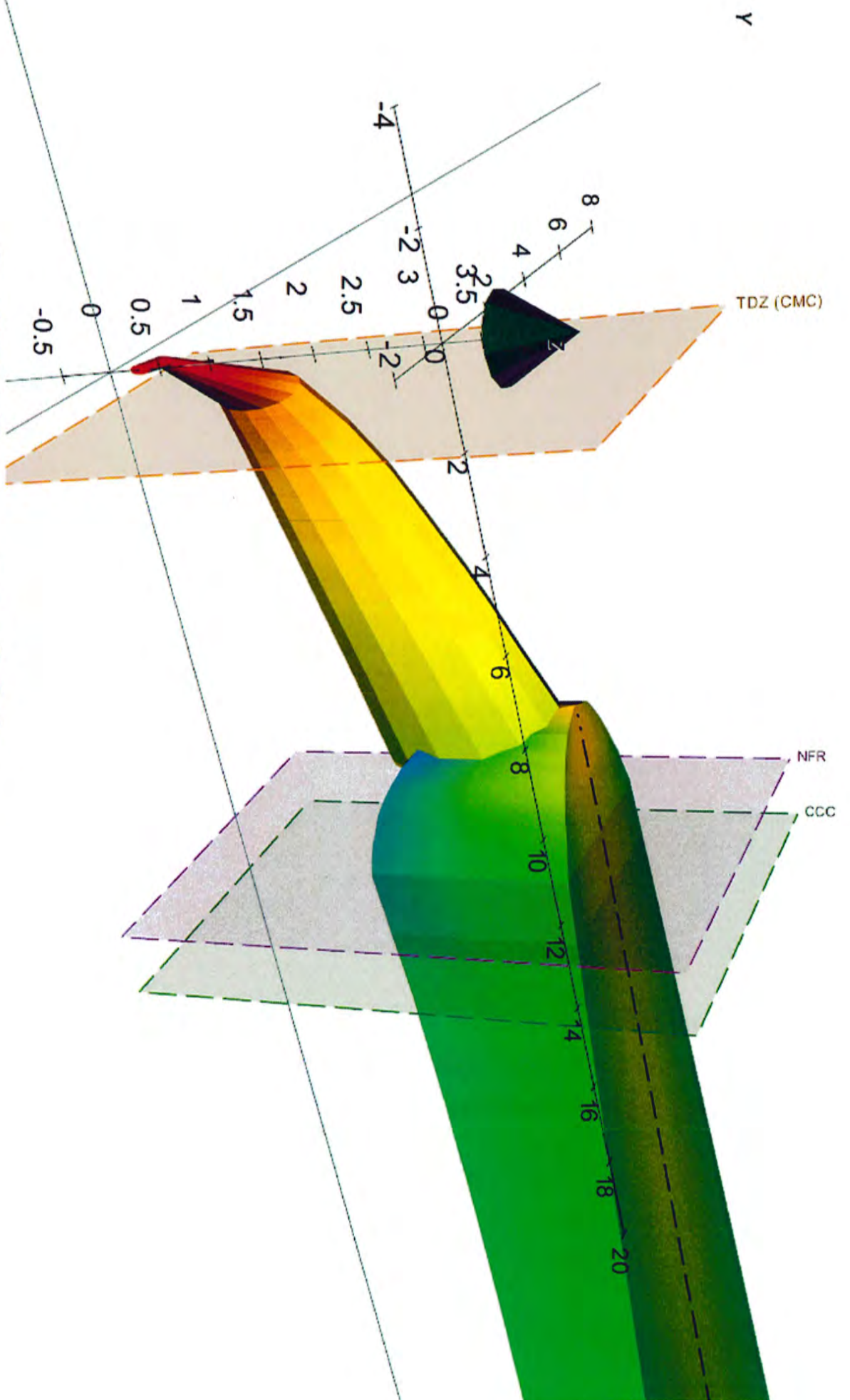
Plume Centrlne
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)



ccess (mg/l)
 0.422 1.077 2.746 7.000

Chronic50thVelocityEbb
 Flow Class: H2
 Origin: Ambient Bottom
 CORMIX1 Simulation
 Length units in meters
 Distortion Scale: Y:X = 1 Z:X = 22
 Visualization up to X = 602 m (out of ROI X = 602 m)

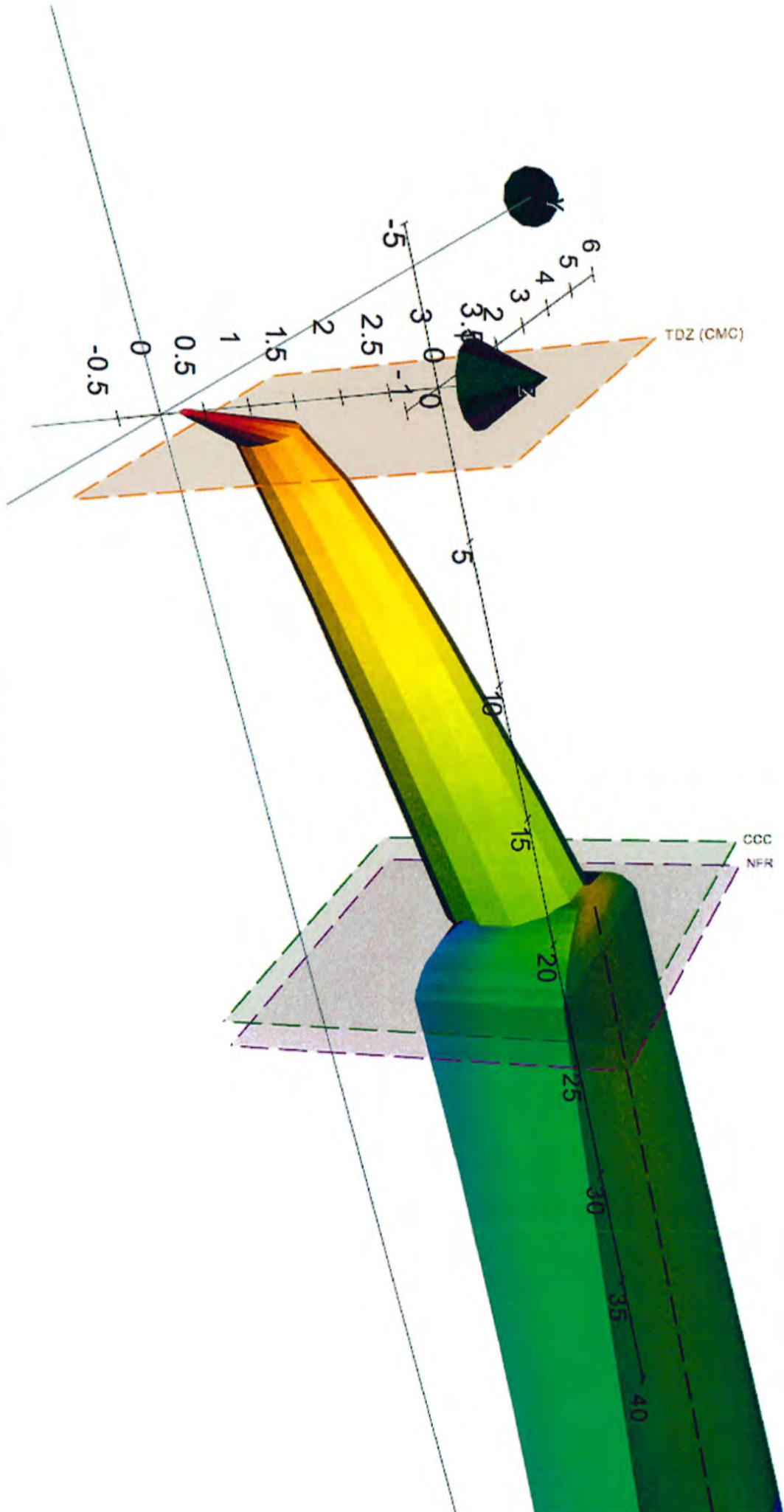
--- Plume Centerline
 --- Toxic Dilution Zone (TDZ - CMC)
 --- Water Quality Standard (WQS - CCC)
 --- End of Near Field Region (NFR)
 --- Cormix Module Boundary (MOD)



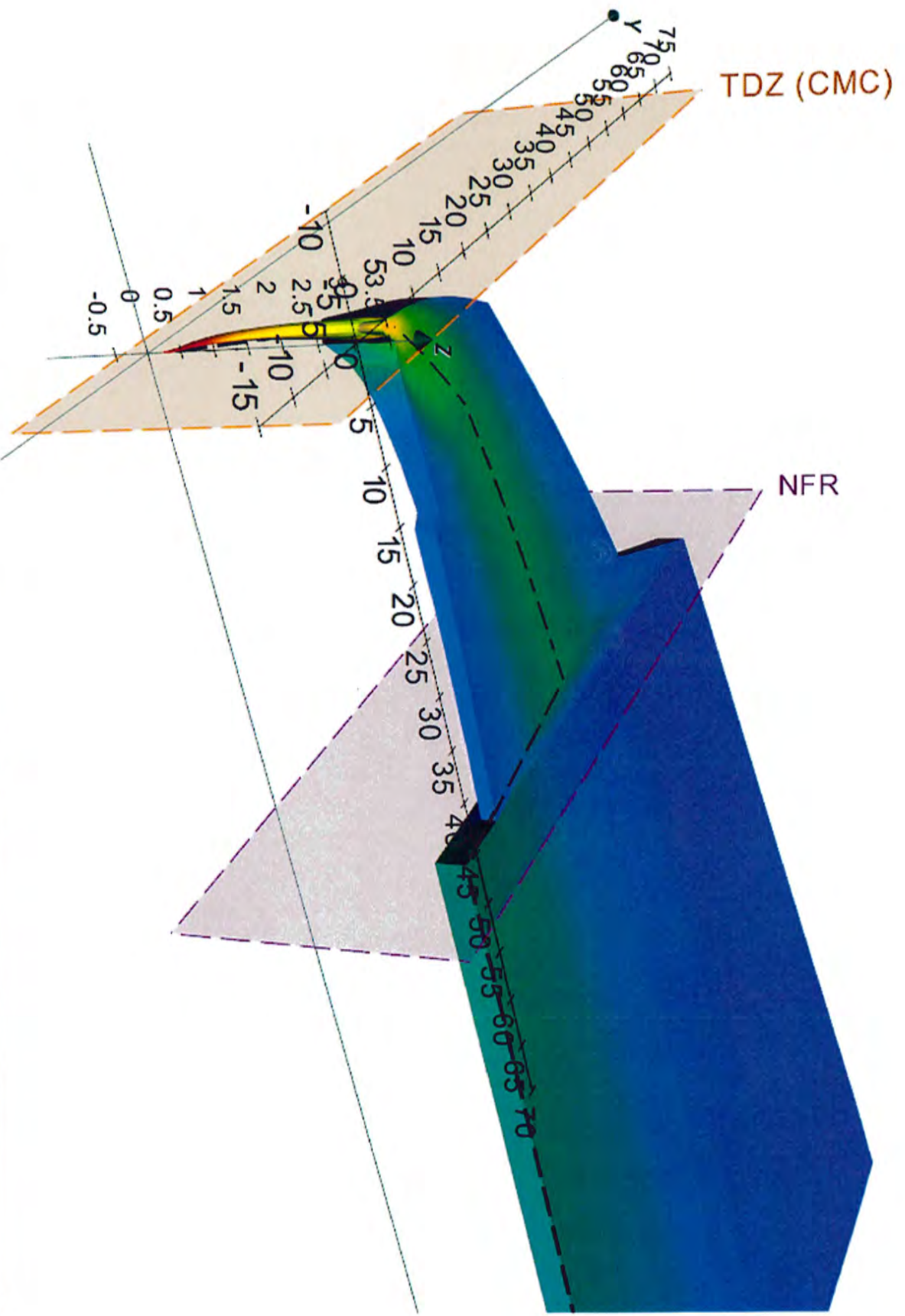
Concentration (mg/l)
 0.422 1.077 2.746 7.000

Accute90thVelocity
 Flow Class: H2
 CO2MIX1 Simulation
 Origin: Ambient Bottom
 Length units in meters
 Distortion Scale: YX = 2.4 ZX = 3.4
 Visualization up to X = 1031 m (out of ROI X = 1031 m)

Plume Cartilage
 Toxic Dilution Zone (TDZ - CMC)
 Water Quality Standard (WQS - CCC)
 End of Near Field Region (NFR)
 Cormix Module Boundary (MOD)



Concentration (mg/l)
 1.13 2.08 3.82 7.00



Accute 10th Velocity
 Flow Class: H4-90
 COORMIX1 Simulation
 Distortion Scale: Y:X = 1 Z:X = 6
 Visualization up to X = 163 m (out of ROI X = 163 m)

Origin: Ambient Bottom
 Length units in meters

- Plume Centerline
- Toxic Dilution Zone (TDZ: CMC)
- End of Near Field Region (NFR)
- Cormix Module Boundary (MOD)

Warnings:
 > Close to Bank/Shore. Boundary interaction at end of near field.



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